Washoe County Development Application

Your entire application is a public record. If you have a concern about releasing personal information, please contact Planning and Building staff at 775.328.6100.

Project Information	**Staff Assigned Case No.:**
Project Name:	Scott's Garage
Project Description:	Garage / work shop
Project Address:	11537 Sitka St. Reno NV. 89506
Project Area (acres or square feet):	2,400 Sq. Ft.
Project Location (with point of reference to major cross streets **AND** area locator):	

Assessor's Parcel No.(s):	Parcel Acreage:	Assessor's Parcel No.(s):	Parcel Acreage:
080-311-04	1.049		

Indicate any previous Washoe County approvals associated with this application:
Case No.(s).

Applicant Information (attach additional sheets if necessary)

Property Owner:		**Professional Consultant:**	
Name:	Scott Hebert	Name:	
Address:	11537 Sitka St	Address:	
	Reno NV. Zip: 89506		Zip:
Phone:	Fax:	Phone:	Fax:
Email:	Scott-hebert@sbcglobal.net	Email:	
Cell:	714 873 5631 Other:	Cell:	Other:
Contact Person:		Contact Person:	
Applicant/Developer:		**Other Persons to be Contacted:**	
Name:		Name:	
Address:		Address:	
	Zip:		Zip:
Phone:	Fax:	Phone:	Fax:
Email:		Email:	
Cell:	Other:	Cell:	Other:
Contact Person:		Contact Person:	

For Office Use Only

Date Received:	Initial:	Planning Area:	
County Commission District:		Master Plan Designation(s):	
CAB(s):		Regulatory Zoning(s):	

Property Owner Affidavit

Applicant Name: Scott Hebert

The receipt of this application at the time of submittal does not guarantee the application complies with all requirements of the Washoe County Development Code, the Washoe County Master Plan or the applicable area plan, the applicable regulatory zoning, or that the application is deemed complete and will be processed.

STATE OF NEVADA)
)
COUNTY OF WASHOE)

I, Scott Hebert,
(please print name)

being duly sworn, depose and say that I am the owner* of the property or properties involved in this application as listed below and that the foregoing statements and answers herein contained and the information herewith submitted are in all respects complete, true, and correct to the best of my knowledge and belief. I understand that no assurance or guarantee can be given by members of Planning and Building.

(A separate Affidavit must be provided by each property owner named in the title report.)

Assessor Parcel Number(s): 080-311-04

Printed Name Scott Hebert

Signed Scott D. Hebert

Address 11537 Sitka St.
Reno, Nv. 89506

Subscribed and sworn to before me this 12th day of July, 2019.

Jeanne Leigh Aharonian
Notary Public in and for said county and state

My commission expires: 09/13/2021

(Notary Stamp)

JEANNE LEIGH AHARONIAN
Notary Public - State of Nevada
County of Washoe
APPT. NO. 17-3574-2
My App. Expires Sept. 13, 2021

*Owner refers to the following: (Please mark appropriate box.)

- [x] Owner
- [ ] Corporate Officer/Partner (Provide copy of record document indicating authority to sign.)
- [ ] Power of Attorney (Provide copy of Power of Attorney.)
- [ ] Owner Agent (Provide notarized letter from property owner giving legal authority to agent.)
- [ ] Property Agent (Provide copy of record document indicating authority to sign.)
- [ ] Letter from Government Agency with Stewardship

Administrative Permit Application
Supplemental Information
(All required information may be separately attached)

1. What is the type of project or use being requested?

Garage / storage / work shop 30x80 2400 sq.ft

2. What section of the Washoe County code requires the Administrative permit required?

Building Bigger than House

3. What currently developed portions of the property or existing structures are going to be used with this permit?

None

4. What improvements (e.g. new structures, roadway improvements, utilities, sanitation, water supply, drainage, parking, signs, etc.) will have to be constructed or installed and what is the projected time frame for the completion of each?

5. Is there a phasing schedule for the construction and completion of the project?

no

6. What physical characteristics of your location and/or premises are especially suited to deal with the impacts and the intensity of your proposed use?

7. What are the anticipated beneficial aspects or effect your project will have on adjacent properties and the community?

Value to Property and clean Yard

8. What will you do to minimize the anticipated negative impacts or effect your project will have on adjacent properties?

Keep building and house Looking good and match Colors

9. Please describe any operational parameters and/or voluntary conditions of approval to be imposed on the administrative permit to address community impacts.

10. How many improved parking spaces, both on-site and off-site, are available or will be provided? (Please indicate on site plan.)

11. What types of landscaping (e.g. shrubs, trees, fencing, painting scheme, etc.) are proposed? (Please indicate location on site plan.)

Paint to match House

12. What type of signs and lighting will be provided? On a separate sheet, show a depiction (height, width, construction materials, colors, illumination methods, lighting intensity, base landscaping, etc.) of each sign and the typical lighting standards. (Please indicate location of signs and lights on site plan.)

13. Are there any restrictive covenants, recorded conditions, or deed restrictions (CC&Rs) that apply to the area subject to the administrative permit request? (If so, please attach a copy.)

☐ Yes	☒ No

14. Utilities:

a. Sewer Service	
b. Water Service	

For most uses, the Washoe County Code, Chapter 110, Article 422, Water and Sewer Resource Requirements, requires the dedication of water rights to Washoe County. Please indicate the type and quantity of water rights you have available should dedication be required:

c. Permit #		acre-feet per year	
d. Certificate #		acre-feet per year	
e. Surface Claim #		acre-feet per year	
f. Other, #		acre-feet per year	

Title of those rights (as filed with the State Engineer in the Division of Water Resources of the Department of Conservation and Natural Resources):

Washoe County Treasurer
Tammi Davis

Washoe County Treasurer
P.O. Box 30039, Reno, NV 89520-3039
ph: (775) 328-2510 fax: (775) 328-2500
Email: tax@washoecounty.us

Bill Detail

Back to Account Detail | Change of Address | Print this Page

Washoe County Parcel Information

Parcel ID	Status	Last Update
08031104	Active	8/14/2019 2:07:35 AM

Current Owner:
HEBERT, SCOTT D
11537 SITKA ST
RENO, NV 89506

SITUS:
11537 SITKA ST

Taxing District
4000

Geo CD:

Legal Description

Township 21 Section 22 Block 6 Lot 11 Range 19 SubdivisionName STREETER SUBDIVISION 1

Installments

Period	Due Date	Tax Year	Tax	Penalty/Fee	Interest	Total Due
INST 1	8/19/2019	2019	$0.00	$0.00	$0.00	$0.00
INST 2	10/7/2019	2019	$0.00	$0.00	$0.00	$0.00
INST 3	1/6/2020	2019	$0.00	$0.00	$0.00	$0.00
INST 4	3/2/2020	2019	$0.00	$0.00	$0.00	$0.00
		Total Due:	**$0.00**	**$0.00**	**$0.00**	**$0.00**

Tax Detail

	Gross Tax	Credit	Net Tax
Remediation	$2.96	$0.00	$2.96
State of Nevada	$67.50	($13.22)	$54.28
Truckee Meadows Fire Dist	$214.42	($41.98)	$172.44
Washoe County	$552.60	($108.20)	$444.40
Washoe County Sc	$452.08	($88.52)	$363.56
Total Tax	**$1,289.56**	**($251.92)**	**$1,037.64**

Payment History

Tax Year	Bill Number	Receipt Number	Amount Paid	Last Paid
2019	2019081885	B19.11391	$1,037.64	7/30/2019

Pay By Check

Please make checks payable to:
WASHOE COUNTY TREASURER

Mailing Address:
P.O. Box 30039
Reno, NV 89520-3039

Overnight Address:
1001 E. Ninth St., Ste D140
Reno, NV 89512-2845

Change of Address

All requests for a mailing address change must be submitted in writing, including a signature (unless using the online form).

To submit your address change online click here

Address change requests may also be faxed to:
(775) 328-3642

Address change requests may also be mailed to: Washoe County Assessor
1001 E 9th Street
Reno, NV 89512-2845

**Not all colors available on certain configurations. Please consult with your representative for details.*

GENERAL NOTES

DIVISION 01 – Section 01 00 00 GENERAL REQUIREMENTS

1. The Contractor shall verify all dimensions and conditions prior to starting construction. The Architect shall be notified of any discrepancies or inconsistencies.
2. Do not scale the Drawings for working dimensions.
3. Notes and details on Drawings shall take precedence over General Notes and Typical Details. Typical details shall apply to the project Drawings except when specific details are shown which shall take precedence.
4. All work shall conform to the minimum standards of the following code:

   The 2018 edition of the International Building Code, and any other regulating agencies which have authority over any portion of the work, and those codes and standards listed in these notes and Specifications.

5. See Architectural or Metal Building Drawings for the following:
   a. Size and location of window and door openings.
   b. Size and location of concrete curbs, floor drains, and depressed areas.
   c. Size and location of floor and roof openings except as shown.
   d. Size and location of interior and exterior non-bearing partitions.

6. See Mechanical, Plumbing, and Electrical Drawings for the following:
   a. Electrical conduit runs, boxes, and outlets in walls, size and location of equipment bases.
   b. Pipe runs, sleeves, hangers, trenches, and openings.
   c. Concrete inserts for fixtures.

7. Contractor shall investigate site during clearing and earth work operations for filled excavations or buried structures such as cesspools, cisterns, foundations, etc. If any such structures are found, notify Structural Engineer immediately.
8. The contract Structural Drawings and Specifications represent the finished structure. They do not indicate the method of construction. The Contractor shall provide all measures necessary to protect the structure during construction. Such measures shall include, but not be limited to, bracing, shoring for loads due to construction equipment, etc. Observation visits to the site by the Structural Engineer shall not include inspection of the above items.
9. Openings, pockets, etc. larger than 6 inches shall not be placed in slabs, decks, beams, joists, columns, walls, etc., Unless specifically detailed on the Structural Drawings. Notify the Structural Engineer when drawings by others show openings, pockets, etc., not shown on the Structural Drawings, but which are located in structural members.
10. Construction materials shall be spread out if placed on framed floors or roof. Load shall not exceed the design live load per square foot. Provide adequate shoring and/or bracing where structure has not attained design strength.
11. Shop Drawings submitted to the Structural Engineer for review shall consist of the number of sets to be returned plus one. Shop drawing submittals shall be bond copies.
12. Adhesive anchors shall be Simpson SET-XP Epoxy per ICBO ESR-2508 with ASTM A36 threaded rod or approved equal, u.n.o. Expansion anchors shall be Simpson Strong Bolts per ICBO-ESR-1771, u.n.o. Adhesive or expansion anchors shall not be installed without authorization by Structural Engineer and until concrete and masonry has cured to design strength.
13. Design loads:
    All loads are per the
    Metal Building Manufacturer.

DIVISION 01 – Section 01 11 00 SUMMARY OF WORK

1. It shall be the contractors direct responsibility to comply with typical details and general notes as delineated or defined on the typical detail drawings of these contract documents regardless of specific flagging or reference to applicable note or detail.
2. It shall be the contractor's responsibility to coordinate with all trades regarding utilities passing through and under footings. Structural requirements for these conditions are delineated in typical details.
3. Top of footing elevations noted are minimum. See note 2 for additional requirements.
4. Contractor to verify and coordinate all locations and sizes of openings in slabs, slab depressions, and curbs for all related construction prior to floor layout or construction. Contractor shall then use appropriate detail(s) or appropriate wall section for each applicable condition.
5. Drawings are diagrammatic in nature and are not intended to indicate every opening or penetration in roof or other structure. Contractor shall coordinate and verify location and size of all such openings and penetrations with related sub-contractors prior to roof or other framing layout or construction. Contractor shall then use appropriate typical or referenced detail(s) for each opening or penetration.
6. Contractor to verify with appropriate sub-contractors the exact location, weight, and intended method of attachment of all items to be suspended from or in any way attached to any roof framing or other structural member unless such item(s) are clearly addressed by the structural construction documents. This information shall be transmitted in writing to the appropriate framing manufacturer via the structural engineer prior to final design or fabrication of structural framing members.
7. Contractor to verify dimensions with architect prior to construction.
8. Contractor to verify all existing conditions and dimensions and notify the architect in writing of any discrepancies.
9. The contractor and all subcontractors he intends to use (including agents and suppliers) are aware of and acknowledge that close coordination among architectural, mechanical, electrical and structural drawings is required for the following:
   a. Determination of all column locations and sizes.
   b. Determination of top of floor, top of steel, wall plate and top of beam elevations.
   c. Verification of all dimensions.
   d. Verify all tops of footings.

10. The contractor and all subcontractors he intends to use (including agents and suppliers) shall make consideration for and include Monies for the above in preparation of their bids. This requirement shall supersede any contained in the AISC "Manual of Steel Construction".

DIVISION 01 – Section 01 82 13 FOUNDATION PERFORMANCE REQUIREMENTS

1. Foundations are spread footings and shall bear on structural fill or native compact granular soil. Foundations have been designed to impose less than 1500 psf in accordance with Table 1806.2 of the International Building Code. Contractor shall verify suitable soil conditions in the field. Should loose, organic or other unsuitable material be encountered, please contact the structural engineer-of-record. Footings are designed based on an allowable soil bearing pressure of 1500 psf with 1/3 increase for short-term loads. All footings shall be 24 inches below adjacent exterior finish Grade.
2. Contractor shall provide for proper de-watering of excavations from surface water, ground water, seepage, etc.
3. Contractor shall provide for design and installation of all cribbing, sheathing and shoring required to safely and adequately retain the earth banks.
4. Excavations for footings shall be approved by the Soils Engineer prior to placing the concrete and reinforcing. Contractor to notify soils Engineer when inspection of excavation is ready. Soils Engineer to submit letter of compliance to the owner.
5. All excavations shall be properly backfilled. Do not place backfill behind retaining walls before concrete has attained full design strength. Contractor shall brace or protect all building and pit walls below Grade from Lateral Loads until attaching floors are completely in place and have attained full strength. Contractor shall provide for design, permits and installation of such bracing.
6. Footings shall be placed and estimated according to depths shown on Drawings.
7. Footing backfill and utility trench backfill within building area shall be mechanically compacted in layers, to the approval of the Soils Engineer. Flooding will not be permitted.
8. All abandoned footings, utilities, etc., that interfere with new construction shall be removed.

DIVISION 01 – Section 01 45 00 SPECIAL INSPECTIONS AND DEFERRED SUBMITTALS

1. Special inspection, per the International Building Code Section 1705.2, for steel and Table 1705.3 for concrete shall be required for the following types of work. See project specifications and Statement of Special Inspections below for specified requirements.

   a. All concrete work for strengths greater than 2500 psi, except for slabs on grade, footings and non structural concrete.
   b. All reinforcing steel for concrete strengths greater than 2500 psi.
   c. All field welding (except metal studs, furring channels, etc.). Shop welding for procedures and multiple pass welds.
   d. All full penetration welds shall be specially inspected in accordance with AWS and the current International Building Code.
   e. All fillet welds shall be visually inspected in accordance with AWS and the current International Building Code.
   f. All masonry work, see notes under 'MASONRY' for requirements. All masonry inspection shall also comply with the National Concrete Masonry Institute.
   g. Bolts installed in conc. or masonry.
   h. All ASTM A-325 and/or ASTM A-490 High Strength Bolts.
   i. All expansion bolts and adhesive anchors.
   j. All grouted dowels.
   k. All insulating concrete.
   L. The metal building manufacturer shall provide their calculations and drawings to the structural engineer of record for review prior to start of construction. The metal building drawings must be stamped and signed by a licensed civil and/or structural engineer in the state of Nevada.

2. Statement of special inspections
   A. This statement of special inspections is accordance with sections 1704.3 of the IBC code. The intent of this section is that all special inspections shall be performed in accordance with the provisions of Chapter 17 of the IBC unless specifically noted otherwise. See structural drawings for other requirements.

   B. The following items require special inspection in accordance with the building code.

   C. Special inspections are not required where the work is done on the premises of a fabricator registered and approved to perform such work without special inspections. Approval shall be based upon review of the fabricators written procedural and quality control manuals and periodic auditing of fabrication practices by an approved special inspection agency. At completion of fabrication, the approved fabricator shall submit a certificate of compliance to the building official stating that the work was performed in accordance with the approved construction documents.

   D. Where special inspection is required to verify the fabricators fabrication and quality control procedures, such inspection shall be performed in accordance with section 1704.2.4 of the IBC.

   E. Special inspection of concrete construction shall conform to the following.
      a. Concrete design mix. provide periodic testing in compliance with table 1705.3 of the IBC. Fresh concrete testing shall include slump, air-content, temperature, and casting of test cylinders for strength testing (see specification for frequency of concrete testing and applicable ASTM).
      b. Periodic inspection of reinforcing steel, including pavement, in compliance with table 1705.3 of the IBC.
      c. Continuous inspection of concrete placement in compliance with table 1705.3 of the IBC.
      d. Periodic inspection of bolts and anchors embedded in concrete prior to the concrete pour.
      e. Post installed anchors in concrete.
      f. Periodic inspection of concrete formwork for shape, location and dimensions of the concrete members being formed in compliance with table 1705.3 of the IBC.
      g. Verification of weldability of reinforcing steel other than ASTM A706 per AWS d1.4 and ACI 318.3.5.2.
      h. Inspection of reinforcing steel welding in accordance with table 1705.2.2.

   F. Special inspection of steel construction shall be per IBC Section 1705.2 in accordance with AISC 360 Chapter N and shall conform to the following.
      a. Special inspection of shop fabrications is required unless the fabricator is registered and approved to perform such work without special inspection.
      b. Continuous visual inspections include:
         1. Complete and partial penetration groove welds.
         2. Multi-pass fillet welds and fillet welds greater then 5/16" (AWS Dd1.1)
         3. Use of proper welding electrodes for "demand critical" welded joints.
      c. Periodic visual special inspections include:
         1. Single pass fillet welds not exceeding 5/16" (AWS D1.1)
         2. Use of proper welding electrodes for non-"demand critical" welded joints.
         3. Bolting materials: bolts, nuts, washers (see specification for applicable ASTM)
         4. Bolt installation for "snug tightened" bearing type connections
         5. Inspection of structural steel frame to verify compliance with the details shown on the approved construction documents, such as bracing, stiffening, member locations and proper application of joint details at each connection.
      d. Continuous testing includes:
         1. All complete-joint and partial-joint penetration groove welded joints shall be tested using approved non-destructive methods conforming to AWS D1.1
      e. Non-destructive testing as referenced above for welding joints may be any of the following methods such that weld discontinuities such as cracks, porosity, incomplete joint penetration, voids, corrosion, slag, etc can be detected: liquid penetrant (PT) ASTM E165, magnetic partical (MT) ASTM E709, radiography (RT) ASTM D94, ultrasonic (UT) ASTM E164.

   G. Material or workmanship not in reasonable conformance with the approved construction documents or specifications may be rejected at any time during the progress of the work.

   H. Special inspection of the seismic-force-resisting system for this building is required.

   I. Testing for seismic resistance are required for reinforcing steel, prestressing steel and structural steel components of the seismic-force resisting system in accordance with Section 1705.12 of the IBC.

   J. See architectural, mechanical, electrical, and plumbing drawings for testing for seismic resistance regarding these systems per Sections 1705.11.5 and 1705.11.6.

   K. The contractor shall request special inspection of the items listed above prior to those items becoming inaccessible and unobservable due to progression of the work.

   L. Special inspector shall provide bi-weekly special inspection reports and shall distribute these reports to the building official, owner, contractor, architect, structural engineer-of-record, and mech/elec/plumbing engineer-of-record. Special inspections reporting shall be in accordance with section 1704.2.4 of the IBC.

DIVISION 03 – Section 03 00 00 CONCRETE

1. All phases of work pertaining to the concrete construction shall conform to the 'Building Code Requirements for Reinforced Concrete' (ACI 318) and the 'Specifications for Structural Concrete for Buildings' (ACI 301) latest approved editions, with modifications as noted in the Drawings or Specifications.
2. Reinforced concrete design is by the 'Ultimate Strength Design method'.
3. Concrete mixes shall be designed by a qualified testing laboratory and approved by the Structural Engineer.
   a. Proposed mix designs shall be no more than 1 (one) year old, and have affixed on each submitted copy the original seal of the Reviewing Engineer. The reviewing Engineer shall be registered in the state of Nevada.
   b. Each mix design shall indicate the project name and address. Contractor shall designate location of use for each proposed mix design.
   c. Each mix design shall include the slump, before and after adding plasticizer, air entrainment, type of aggregate, type of cement, and admixtures to be used.
   d. All exposed concrete shall have air entrainment.
   e. No calcium chloride shall be used.
   f. Water cement ratio for footings shall not exceed 0.50.
   g. Slab on grade shall have a water cement ratio of 0.45 and shall be moisture cured per ACI 318 Section 5.11 requirements.
   h. Concrete may have a maximum of 15% fly ash substitution for cement. Verify with Architect.
   i. An approved curing compound compatible with the stain finish can be used.

4. Schedule of Structural concrete 28-day strengths and types:

LOCATION IN STRUCTURE	STRENGTH PSI	TYPE
Footings:	2500	Normal Wt. 145 ± 5 pcf
Slabs on grade:	4000	Normal Wt. 145 ± 5 pcf

Section 03 00 00 CONCRETE CONTINUED

5. Portland cement shall conform to ASTM C-150, type II. Use minimum 5.5 sacks cement/c-y and maximum 3" slump with water (slump may be increased with admixtures that do not promote shrinkage). Provide 6% ± 1% air entrainment in concrete exposed to weather.
6. Maximum aggregate size shall conform with the following: 1/5 distance between forms, 3/4 distance between reinforcing bars, 1/3 thickness of slab.
   a. Aggregate for hard rock concrete shall conform to all requirements and tests of ASTM C-33 and project Specifications. Exceptions may be used only with permission of the Structural Engineer.
7. Forms for elevated concrete beams shall be laid out and constructed to provide the specified cambers shown on the drawings
8. Dry pack under base plates, sill plates, etc., see Specifications.
9. Concrete mixing operations, etc., shall conform to ASTM C-94.
10. Placement of concrete shall conform to ACI-318 requirements.
11. If columns and wall are placed with floor. two hours must elapse between end of column or wall pour and beginning of floor pour.
12. Clear coverage of concrete over outer reinforcing bars shall be as follows:

    a. Concrete poured directly against earth: 3 in. clear to reinforcing.
    b. Formed concrete with earth backfill: 2 in. clear.
    c. Slabs on Grade: center in slab.

13. All reinforcing bars, anchor bolts and other concrete inserts shall be well secured in position prior to placing concrete.
14. Provide sleeves for plumbing and electrical openings in concrete before placing. Do not cut any reinforcing which may conflict. Coring in concrete is not permitted except as shown. Notify the Structural Engineer in advance of conditions not shown on the Drawings.
15. Conduit shall not be placed in slabs or walls unless specifically detailed otherwise.
16. Projecting corners of beams, walls, columns, etc., shall be formed with a 1/2 in. chamfer, unless otherwise noted on Architectural Drawings.
17. Curing compounds used on concrete that is to receive a resilient tile finish shall be approved by the tile manufacturer before use.
18. Place and protect concrete in compliance with ACI 305 and 306, respectively, during hot and cold exposure conditions.

DIVISION 03 – Section 03 21 00 REINFORCING STEEL

1. All reinforcing steel shall be detailed and placed in conformance with the 'Building Code Requirements for Reinforced Concrete' (ACI 318 latest approved edition), and the 'Manual of Standard Practice for Reinforced Concrete Construction' (latest edition) by the C.R.S.I. and the W.C.R.S.I., as modified by the project Drawings and Specifications.
2. Deformed reinforcing bars shall be ASTM A-615 Grade 60 except ties, stirrups, slab dowels and reinforcing bars in non structural concrete such as slabs on grade, which may be Grade 40, unless noted otherwise. Use A706 reinforcing bars that are required for welding.
3. Welding of reinforcing shall be with low hydrogen electrodes in conformance with 'Recommended Practices for Welding Reinforcing Steel, etc.', American Welding Society, AWS-D1.4. See Specifications.
4. All reinforcing bar bends shall be made cold.
5. Welded wire fabric shall conform to ASTM A-185.
6. Minimum lap of welded wire fabric shall be 6 inches or one full mesh and one half, which ever is greater.
7. Reinforcing splices shall be made only where indicated on the drawings.
8. Dowels between footings and walls or columns shall be the same grade, size and spacing or number as the vertical reinforcing, respectively.
9. All bars shall be marked so their identification can be made when the final in-place inspection is made.
10. Splice reinforcing bars per detail 5/S0.2
11. All reinforcing bars to be tied in place before pouring concrete or grout.
12. Do not splice reinforcing steel in middle third of walls.

DIVISION 05 – Section 05 12 00 STRUCTURAL STEEL FRAMING

1. Structural steel shall be detailed, fabricated and erected in accordance with the AISC Specifications for the design, fabrication and erection of Structural steel for buildings (latest edition and supplements).
2. All Structural steel shall conform to ASTM A-992 with fy=50 ksi, unless noted otherwise. Misc. steel such as Plates, and Angles may be ASTM-A36.
3. Pipe columns shall conform to ASTM designation A-53 Grade 'B'. All steel tubes shall conform to ASTM A-500 Grade 'B' cold formed tubes with fy = 46 ksi. unless noted otherwise on plans.
4. All bolts, except anchor bolts, shall conform to ASTM A-325, connection type N, Anchor bolts shall conform to ASTM A-307 A36 or F1554. grade 36 unless noted otherwise. All bolts shall have a minimum of 3 threads projecting beyond the nut.
5. Structural steel fabricator shall furnish shop drawings of all Structural steel, respectively, for Architect's and Engineer's review before fabrication.
6. Bolt holes in steel shall be 1/16 inch larger than nominal size of bolt used, except anchor bolt holes for column base plates which may be 3/16 inch larger.
7. All Structural steel surfaces shall be shop painted. All steel exposed to weather shall have two coats of paint.
8. All welds shall be in conformity with the Structural welding code (AWS D1.1) of the American welding society. See I.B.C.
9. Weld lengths called for on plans are the net effective length required. Use E70XX electrodes.
10. Welding tests and inspections, see I.B.C.

DIVISION 05 – METAL BUILDING

1. Foundations per Sheet S1.1 have been designed for the reactions provided by Olympia Steel Buildings, Job Number 004989, Dated 6/7/19 and Stamped/Signed 06/17/2019.

2. The metal building shall be designed to support all the components and equipment shown on the Architectural, Electrical, Mechanical and Structural Drawings. The anchor bolt size and pattern (layout) along with the base plate shall be designed by a metal building manufacturer and approved by the engineer of record prior to start of construction.

DIVISION 07 – Section 07 26 00 UNDER-SLAB VAPOR BARRIER

1. Under slab vapor barrier to be installed per manufacturer's recommendations and ASTM E 1643-04.

   a. Unroll Vapor Barrier with the longest dimension parallel with the direction of the pour.
   b. Lap Vapor Barrier over footings and seal to foundation walls.
   c. Overlap joints 6 inches and seal with manufacturer's tape.
   d. Seal all penetrations (including pipes) per manufacturer's instructions.
   e. No penetration of the Vapor Barrier is allowed except for reinforcing steel and permanent utilities.
   f. Repair damaged areas by cutting patches of Vapor Barrier, overlapping damaged area 6 inches and taping all four sides with tape.

2. Water vapor transmission rate (WVTR) shall be less than .006 (.012 perms) as tested by ASTM E 96.
3. Vapor barrier shall be not less than 10 mils thick and comply with ASTM E 1745 Class A and ACI 302.1R.

SHEET INDEX

ABBREVIATIONS:

ADDITIONAL	ADD'L	INFORMATION	INFO
ALTERNATE	ALT	INSIDE DIAMETER	ID
ANCHOR BOLT	A.B.	INTERIOR	INT
APPROXIMATE	APPROX	ISOLATION JOINT	IF
AT	@, AT	JOIST	JST
BEAM	BM	KILN DRIED	KD
BEARING	BRG	KING STUD	KING
BELOW	BEL	LAMINATED VENEER LUMBER	LVL
BETWEEN	BET	LIGHT	LT
BLOCK	BLK	LIVE LOAD	LL
BOTH SIDES	B/S	LONG	LG
BOTTOM	BOT	LONG LEG HORIZONTAL	LLH
BOUNDARY NAILING	B.N.	LONG LEG VERTICAL	LLV
BUILDING	BLDG	MACHINE BOLT	MB
CANTILEVER	CANT	MALLEABLE IRON WASHER	MIW
CARRIAGE BOLT	C.B.	MANUFACTURER	MFR
CEILING	CLG	MAXIMUM	MAX
CENTERLINE	℄	MECHANICAL	MECH
CHANNEL	CHNL	MICRO-LAM (BY TRUS JST)	ML
CLEAR	CLR	MINIMUM	MIN
COLUMN	COL	MISCELLANEOUS	MISC
COMPLETE PENETRATION	CP	NEW	(N)
CONCRETE	CONC.	NOT IN CONTRACT	N.I.C.
CONCRETE MASONRY UNIT	CMU	NOT TO SCALE	NTS
CONTINUOUS	CONT	NUMBER/POUNDS	#
CONTROL JOINT	CJ	ON CENTER	O.C.
CONTROL MASONRY JOINT	C.M.J.	OPPOSITE	OPP
COUNTERSINK	C/S	OPPOSITE HAND	O/H
DEAD LOAD	D.L.	ORIENTED STRAND BOARD	O.S.B.
DETAIL	DET	OUTSIDE DIAMETER	OD
DIAMETER	ø, DIA.	OVER	o/
DIMENSION	DIM	PARALLAM (BY TRUS JST)	PSL
DITTO	DO	PARALLEL	PARL or //
DOWEL JOINT	DJ	PARTIAL PENETRATION	PP
DOUBLE	DBL	PENETRATION	PEN
DOUGLAS FIR	DF	POUNDS PER SQUARE FOOT	PSF
DRAWING	DWG	POUNDS PER SQUARE INCH	PSI
EACH	EA	PLATE	PL
EACH END	EE	PLYWOOD	PLY
EACH FACE	EF	POWER DRIVEN FASTENER	PDF
EACH SIDE	ES	PRESSURE TREATED or	
EACH WAY	EW	PRESERVATIVE TREATED	PT
EDGE NAIL	E.N.	PROPERTY LINE	PL
ELEVATION	ELEV	RADIUS	R.
EMBEDMENT	EMBED	REDWOOD	RWD
EQUAL	EQ	REFERENCE	REF.
EXISTING	(E)	REQUIRED	REQ'D
EXPANSION	EXP	ROSBORRO MFG. TIMBER	RMT.
EXPANSION BOLT	E.B.	SCHEDULE	SCHED
EXPANSION JOINT	EJ	SEE ARCHITECTURAL DWGS	SAD
EXTERIOR	EXT	SEE MECHANICAL DWGS	SMD
FACE OF CONCRETE	F.O.C.	SELF-TAPPING SCREW	STS
FACE OF MASONRY	F.O.M.	SHEAR WALL	SW
FACE OF STUD	F.O.S.	SIMILAR	SIM
FIELD NAIL/FACE NAIL	F.N.	SLAB JOINT	SJ
FINISH	FIN	SLAB ON GRADE	S.O.G.
FLOOR	FLR	SOLID BLOCK	SB
FOOTING	FTG	SPECIFICATION	SPEC
FACE OF CONCRETE	F.O.C.	SQUARE	SQ
FACE OF MASONRY	F.O.M.	STANDARD	STD
FACE OF STUD	F.O.S.	STEEL	STL
FIELD NAIL/FACE NAIL	F.N.	SYMMETRICAL	SYM
FINISH	FIN	THREADED	THD
FLOOR	FLR	TOE NAIL	T.N.
FOOTING	FTG	TONGUE & GROOVE	T&G
FORCED-ENTRY FASTNERS	FEF	TOP & BOTTOM	T&B
FOUNDATION	FDN	TOP OF	T.O.
GAGE	GA	TUBE STEEL	TS
GALVANIZED	GALV	TRIMMER	TRMR.
GLU-LAM	G.L.	TYPICAL	TYP
GLUED-LAMINATED BEAM	G.L.B.	UNIFORM BUILDING CODE	UBC
GYPSUM BOARD	GYP BD	UNLESS NOTED OTHERWISE	UNO
HANGER	HGR	VERIFY IN FIELD	VIF
HEADER	HDR	VERTICAL	VERT
HEIGHT	HT	WEAKENED PLANE JOINT	WPJ
HEM-FIR	HF	WEIGHT	WT
HIGH-STRENGTH BOLT	HSB	WELDED STUD/WOOD SCREW	WS
HORIZONTAL	HORIZ	WELDED WIRE FABRIC	WWF
		WELDED WIRE MESH	WWM
		WITH	w/

REVISIONS		
Δ	Revision Type	By

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STRUCTURAL ONLY

7/10/19

HEBERT METAL
BUILDING FOUNDATION
11537 SITKA STREET
RENO, NEVADA 89506
WASHOE COUNTY

DRAWN BY	T.E.S.
CHECKED BY	R.C.C.
DATE	7/10/19
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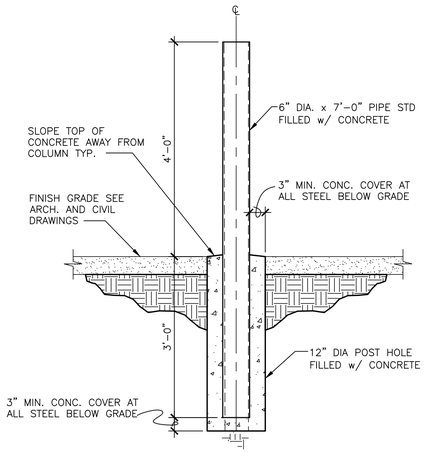
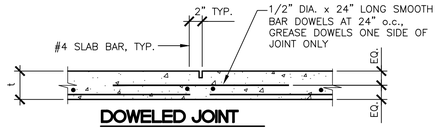
SHEET	S0.1
NUMBER OF SHEETS	



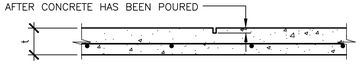
12 DETAIL

N.T.S.

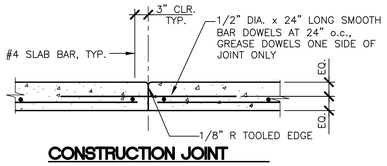


DOWELED JOINT

SAW CUT 1/8" WIDE CONTRACTION JOINT TO A DEPTH OF t/4 MIN., SAWING MUST OCCUR AS SOON AS CONCRETE SURFACE IS FIRM ENOUGH SO CONCRETE WILL NOT BE DAMAGED, BUT NO LATER THAN 12 HOURS AFTER CONCRETE HAS BEEN POURED



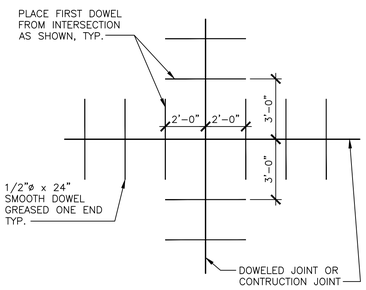
CONTRACTION JOINT



CONSTRUCTION JOINT

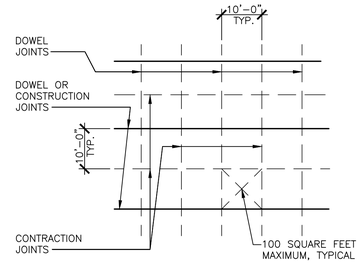
11 JOINTS IN SLAB ON GRADE

0_09C.01 / N.T.S.



10 DOWELING AT SLAB JOINTS

0_07C.01 / N.T.S.



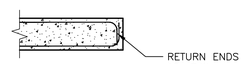
NOTES:
1. SLAB SHALL BE POURED IN STRIP PATTERN. STRIPS MAY BE AS LARGE OF POURS AS PRACTICAL.
2. CONTRACTORS SHALL OBTAIN ARCHITECT'S APPROVAL FOR ALL JOINT LOCATIONS, POUR METHOD AND SIZE.
3. CONTROL JOINTS SHALL BE LOCATED AT AREAS THAT INDUCE CRACKING SUCH AS, BUT NOT LIMITED TO, RE-ENTRANT CORNERS, COLUMNS, WALLS, ETC.

9 METHOD OF POURING SLAB ON GRADE

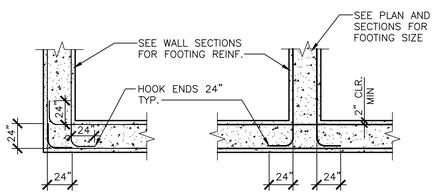
0_08C.02 / N.T.S.

8 NOT USED

N.T.S.



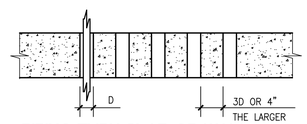
C FOOTING END



A FOOTING CORNER **B FOOTING INTERSECTION**

7 TYP. REINFORCED LAP IN FOOTINGS

0_06C.01 / N.T.S.

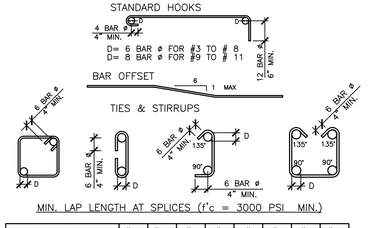


PIPING THRU SLAB OR WALL

NOTES:
1. WHERE CLEAR DISTANCE BETWEEN SLEEVES IS IMPOSSIBLE THIS AREA SHALL BE TREATED AS A SLAB OPENING OR AS A WALL OPENING
2. DO NOT RUN CONDUIT HORIZONTALLY THROUGH SLAB OR WALLS. PLACE IN BASE MATERIAL.

6 PIPING AND CONDUIT THRU SLAB OR WALL

N.T.S.



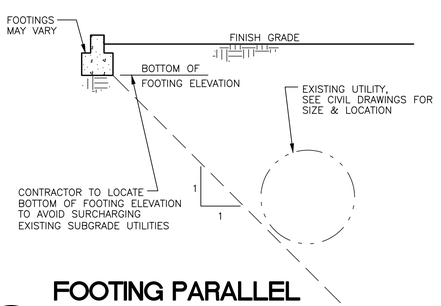
MIN. LAP LENGTH AT SPLICES (f'c = 3000 PSI MIN.)

SIZE OF REINF.	#3	#4	#5	#6	#7	#8	#9	#10
CONCRETE	24"	30"	46"	65"	89"	117"	148"	188"

5 REBAR CONFIG. AND LAPS

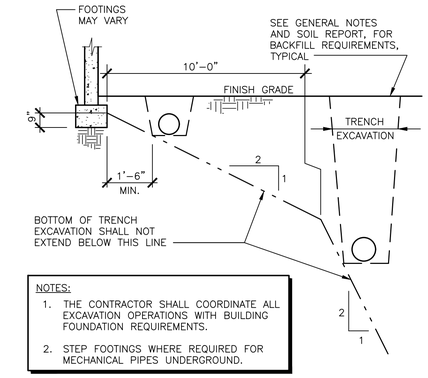
0_04C.01 / N.T.S.

IT SHALL BE THE CONTRACTORS DIRECT RESPONSIBILITY TO COMPLY WITH TYPICAL DETAILS AND GENERAL NOTES AS DELINEATED OR DEFINED ON THE TYPICAL DETAIL DRAWINGS OF THESE CONTRACT DOCUMENTS REGARDLESS OF SPECIFIC FLAGGING OR REFERENCE TO APPLICABLE NOTE OR DETAIL.



4 FOOTING PARALLEL TO EXISTING UTILITY

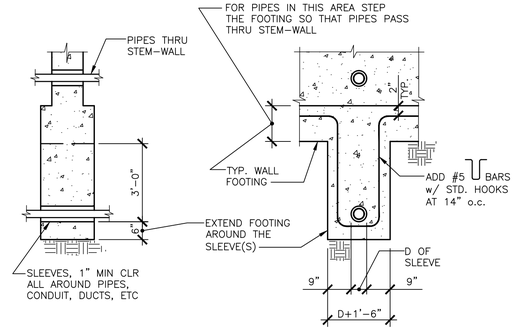
N.T.S.



NOTES:
1. THE CONTRACTOR SHALL COORDINATE ALL EXCAVATION OPERATIONS WITH BUILDING FOUNDATION REQUIREMENTS.
2. STEP FOOTINGS WHERE REQUIRED FOR MECHANICAL PIPES UNDERGROUND.

3 EXCAVATION PARALLEL TO FOOTING

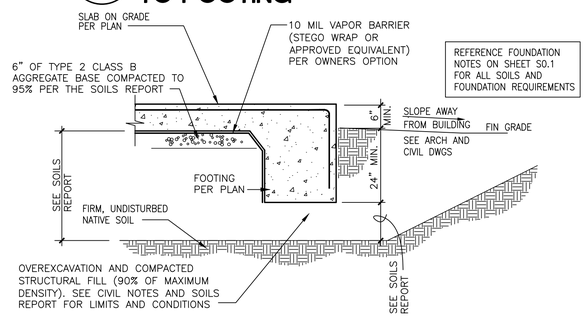
0_03C.01 / N.T.S.



NOTE:
FOR PIPES 3'-0" OR LESS BELOW BOTTOM OF FOOTINGS PROVIDE SLEEVE AND CONCRETE AS SHOWN; MORE THAN 3'-0" USE STEPPED FOOTING TO STAY WITHIN 3'-0" LIMIT

2 EXCAVATION PERPENDICULAR TO FOOTING

0_02C.01 / N.T.S.



NOTES:
1. PROVIDE COMPACTED STRUCTURAL FILL BENEATH CONTINUOUS AND SPREAD FOOTINGS (SEE CIVIL NOTES AND SOILS REPORT). SEE THE PROJECT CIVIL NOTES AND SOILS REPORT FOR ALL SOILS REQUIREMENTS INCLUDING COMPACTION, FILL, AND ALL OTHER REQUIREMENTS. THE BOTTOM OF FOOTING, STRUCTURAL FILL AND COMPACTION SHALL BE REVIEWED AND APPROVED BY THE SOILS ENGINEER.
2. SEE PROJECT SOILS REPORT BY SOILS ENGINEER, GRADING PLANS BY CIVIL ENGINEER AND SHEET S0.1 FOR FILL AND OTHER REQUIREMENTS

1 TYPICAL SOIL PREPARATION

N.T.S.

REVISIONS		
Δ	Revision Type	By

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STRUCTURAL ONLY

STATE OF NEVADA · ROBERT C. CORBIN · EXP. 12-31-20 CIVIL · No. 21379 · REGISTERED PROFESSIONAL ENGINEER

7/10/19

HEBERT METAL
BUILDING FOUNDATION
11537 SITKA STREET
RENO, NEVADA 89506
WASHOE COUNTY

DRAWN BY	T.E.S.
CHECKED BY	R.C.C.
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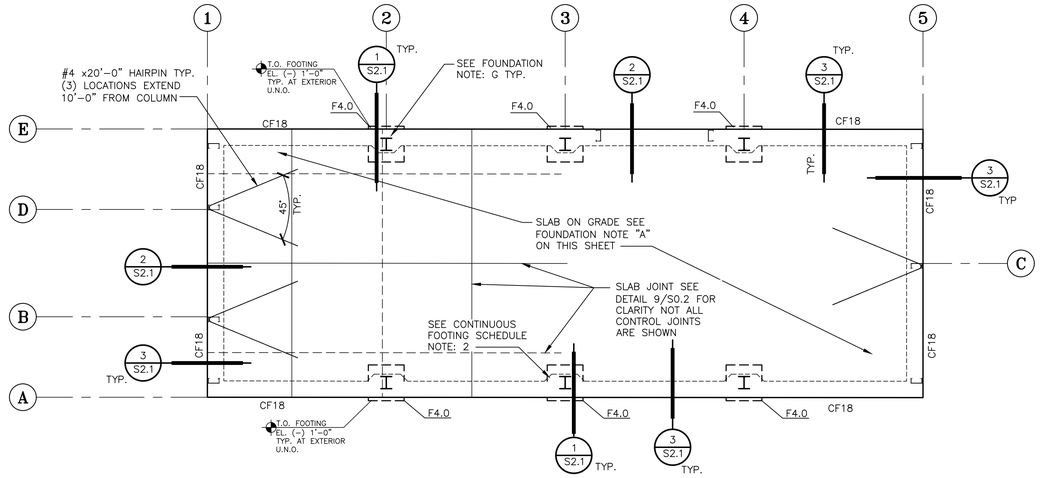
NUMBER OF SHEETS	



FOUNDATION PLAN

1/8"=1'-0"

LEGEND

FX.X — SPREAD FOOTING, SEE SCHEDULE ON THIS SHEET

CFXX — CONTINUOUS FOOTING, SEE SCHEDULE ON THIS SHEET

T.O. FOOTING EL. (-) 0'-0" — INDICATES TOP OF FOOTING

FOUNDATION NOTES:

A. TOP OF SLAB ON GRADE SHALL BE REFERENCE ELEVATION 0'-0", TYPICAL, UNLESS NOTED, THUS (-)x'-x". SLAB ON GRADE SHALL BE 5" THICK CONCRETE WITH #4 AT 18" o.c. EACH WAY, CENTERED IN SLAB, OVER 6" TYPE 2 AGGREGATE BASE. FOR METHOD OF POURING SLAB ON GRADE, SEE DETAILS 9, 10 & 11/S0.2. SEE ARCHITECTURAL DRAWINGS FOR SLAB SLOPES, DEPRESSIONS, ETC...DO NOT PLACE CONDUIT IN SLAB.

B. COORDINATE AND VERIFY ALL DIMENSIONS WITH THE METAL BUILDING MANUFACTURER DRAWINGS. CONTACT METAL BUILDING MANUFACTURER WITH DISCREPANCIES BEFORE CONSTRUCTION OCCURS.

C. SEE GENERAL NOTES AND STRUCTURAL DETAILS FOR BALANCE OF INFORMATION.

D. FOR EXCAVATION SEE DETAILS 1, 2 AND 3/S0.2

E. SEE DETAIL 1/S0.2 FOR TYPICAL SOILS PREPARATION FOR FOUNDATIONS AND SLAB ON GRADE. A QUALIFIED SOILS ENGINEER SHALL INSPECT ALL EXCAVATIONS TO VERIFY ADEQUATE OVEREXCAVATION LIMITS AND SHALL PROVIDE A LETTER TO THE OWNER CONFIRMING SOIL CONDITIONS. SEE SOILS REPORT FOR ADDITIONAL REQUIREMENTS.

F. VERIFY EXTENT AND DEGREE OF SLOPED FLOOR WITH OWNER.

G. COORDINATE ANCHOR BOLTS WITH METAL BUILDING SUPPLIER AND DETAIL 1/S2.1 . ALL ANCHOR BOLTS SHALL HAVE A MINIMUM EMBEDMENT DEPTH OF 6".

METAL BUILDING NOTES AND REQUIREMENTS

REFER TO THE METAL BUILDING MANUFACTURER DRAWINGS REGARDING THE METAL BUILDING FRAMING. THE METAL BUILDING MANUFACTURER IS RESPONSIBLE FOR COORDINATING THE FOOTINGS SHOWN WITH THERE COLUMN, BASE PLATE AND ANCHOR BOLT LOCATIONS AND LAYOUT. THE METAL BUILDING MANUFACTURER SHALL PROVIDE ROOF FRAMING TO SUPPORT ALL LOADS SPECIFIED ON THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND STRUCTURAL DRAWINGS. COORDINATE AND VERIFY ALL DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS. CONTACT THE ENGINEER WITH DISCREPANCIES BEFORE CONSTRUCTION OCCURS.

NOTE:
CONTRACTOR TO COORDINATE ALL SLAB PENETRATIONS AND SLOPED FLOOR LOCATIONS w/ARCH, MECHANICAL AND PLUMBING DRAWINGS WHETHER SHOWN OR NOT

CONTINUOUS FOOTING SCHEDULE

MARK	SIZE (WIDTH x DEPTH)	CONTINUOUS REINFORCEMENT	REMARKS
CF18	1'-6" x PER DETAILS	(2) #5 TOP AND BOTTOM	1. CONTINUE REINFORCING THROUGH SPREAD FOOTING. 2. INCREASE FOOTING WIDTH AT ANCHOR BOLTS AS REQUIRED FOR 3" MIN. CONCRETE COVERAGE

SPREAD FOOTING SCHEDULE

MARK	SIZE (W X L X D)	REINFORCEMENT
F4.0	4'-0"x4'-0"x24"	(4) #5 EACH WAY TOP AND BOTTOM

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PROFESSIONAL ENGINEER STATE OF NEVADA
ROBERT C. CORBIN
No. 21379
EXP. 12-31-20
CIVIL

7/10/19

HEBERT METAL BUILDING FOUNDATION

11537 SITKA STREET
RENO, NEVADA 89506
WASHOE COUNTY

DRAWN BY	T.E.S.
CHECKED BY	R.C.C.
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SHEET	S1.1
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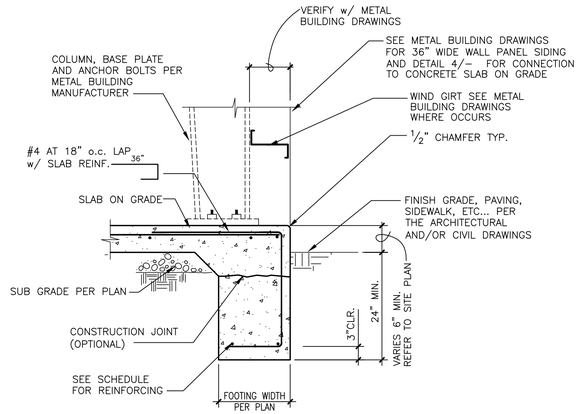
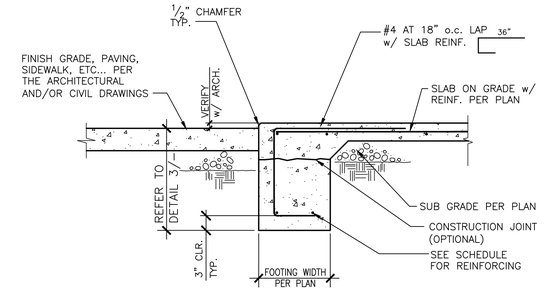


3 EDGE OF SLAB

N.T.S.



2 DETAIL

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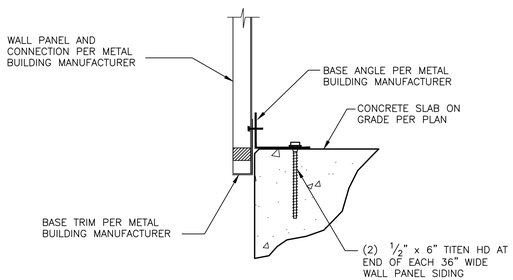
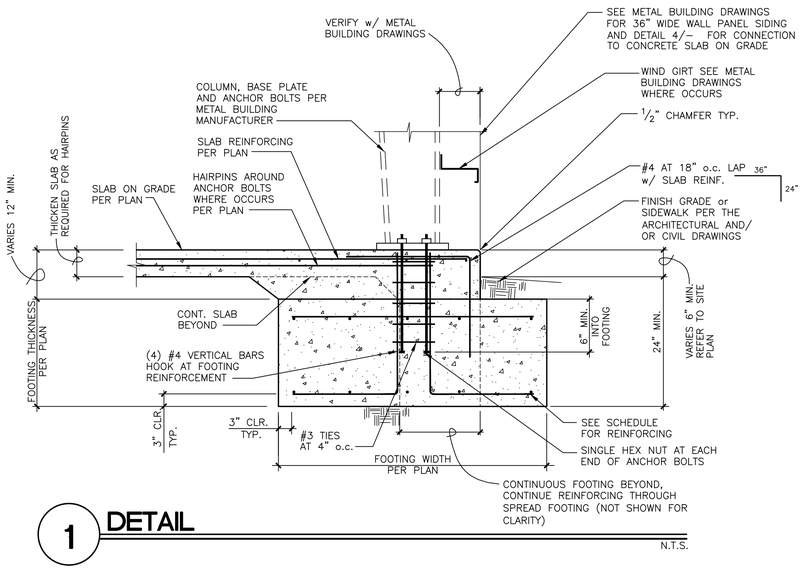


4 BASE TRIM SECTION

N.T.S.



1 DETAIL

N.T.S.

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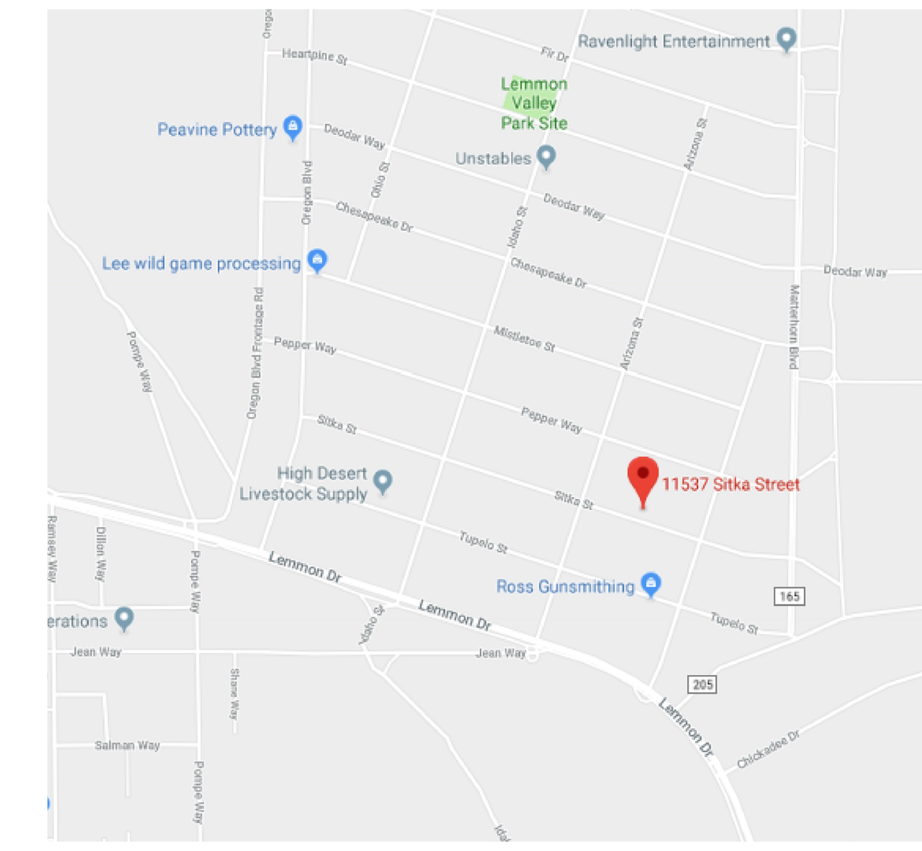
SHEET	S2.1
NUMBER OF SHEETS	

SITE PLAN

1" = 20'-0"



Vicinity Map

N.T.S.

SITE PLAN NOTES:

1. DIMENSIONS TO EXISTING AND PROPOSED STRUCTURES ARE ESTIMATES TO THE NEAREST ONE FOOT INTERVAL, PROPERTY WAS NOT SURVEYED. SITE INFORMATION DERIVED FROM **WASHOE COUNTY ASSESSOR PROPERTY DATA**, ONLINE GIS INTERNET MAP SERVER, EXISTING DOCUMENTS AND SITE VISIT..
2. EXISTING LANDSCAPING, GRADING AND DRAINAGE TO REMAIN. DO NOT DISTURB DURING CONSTRUCTION.
3. PROJECT ENGINEER ASSUMES NO RESPONSIBILITY FOR ACCURACY, FUNCTION OR COMPLIANCE OF EXISTING SEPTIC SYSTEMS AS SHOWN ON THIS SITE.
4. NO DOCUMENTED EXISTING WELLS ON ADJACENT PROPERTIES LOCATED WITHIN 200' OF THE SUBJECT PROPERTY. PROPERTY HAS SEPTIC PER PLAN.
5. NO DOCUMENTED EXISTING SEPTIC SYSTEMS LOCATED WITHIN 200' OF THE SUBJECT PROPERTY BASED UPON INFORMATION SUPPLIED BY **WASHOE COUNTY HEALTH DEPARTMENT**.
6. NO DOCUMENTED NATURAL DRAINAGES NOTED ON SITE OR WITHIN 100' OF SUBJECT PROPERTY.
7. PROPERTY IS OUTSIDE FEMA FLOOD ZONE DESIGNATIONS AND NO 1OO YEAR FLOOD PLAIN BOUNDARIES WITHIN 100' OF PROPERTY.

PROJECT SITE NOTES:

1. CONTRACTOR SHALL PROVIDE **2%** SLOPE DRAINAGE AWAY FROM EXISTING AND NEW CONSTRUCTION MIN. 10' FROM STRUCTURES.
2. CONTRACTOR SHALL VERIFY LOCATION OF EXISTING SEPTIC SYSTEM AND VERIFY COMPLIANCE WITH WASHOE COUNTY HEALTH DEPARTMENT GUIDELINES AND APPLICABLE BUILDING CODES.

SITE PLAN

FOR:

SCOTT HEBERT

11537 SITKA ST.
RENO, NV 89506
APN: 080-311-04

7/10/2019 2:28:03 PM

SIGNATURE OF QUALIFIED INDIVIDUAL OR REPRESENTATIVE

OLYMPIA STEEL BUILDINGS

BUILDING SPECIFICATIONS

The manufacturer is not responsible for the concrete foundation design. The structure under this contract has been designed and detailed for the loads and conditions stipulated in the contract and shown on these drawings. Any alterations to the structural system or removal of any component parts, or the addition of other construction materials or loads must be done under the advice and direction of a registered architect, civil or structural engineer. The manufacturer will assume no responsibility for any loads not indicated.

This manufactured building is designed with the manufacturer's standard design practices which are based on pertinent procedures and recommendations of the following organizations and codes :

- –American Institute of Steel Construction "Specification for the design fabrication and erection of structural steel for buildings.".
- –American Iron and Steel Institute "Specification for the design of cold formed steel structural members" 2007 edition.
- –Metal Building Manufacturers Association "Specification for the design fabrication and erection of the structural system" most current edition.

Material properties of steel plate and sheet used in fabrication of primary rigid frames and all primary structural framing members (other than cold–formed sections) conform to ASTM A–529 or A–572 all with a minimum yield point of 55 KSI.

Material properties of cold formed light gage steel members conform to the requirements of ASTM A–653, with a minimum yield point of 55 KSI

High strength bolts and their installation shall conform to ASTM specification A–325 and are designed as bearing type connections with threads included in the shear plane. All high strength bolts are to be installed to the "Snug–Tight" condition as defined by the RCSC Specification for Structural Joints Using A325 or A490 Bolts, Latest Edtion, section 8.1, unless noted otherwise. Bolts in standard holes do not require washers per section 6.

Shop and field inspections and associated fees are the responsibility of the contractor, unless stipulated otherwise.

CONTRACTOR RESPONSIBILITIES

The contractor must secure all required approvals and permits from the appropriate agency as required.

Approval of the manufacturer's drawings and calculations indicate that the manufacturer has correctly interpreted and applied the requirements of the contract drawings and specifications. (AISC 303–10 Code of Standard Practice)

Where discrepancies exist between the manufacturer's structural steel plans and the plans for other trades, the structural steel plans shall govern. (Section 3.3 AISC 303–10 Code of Standard Practice)

Design considerations of any materials in the structure which are not furnished by the manufacturer, are the responsibility of the contractor and engineers other than the manufacturer's engineering, unless specifically indicated.

The contractor is responsible for all erection of steel and associated work in compliance with the manufacturer's "For Construction" drawings.

Temporary supports, such as guys, braces, flashwork or other elements required for the erection will be determined and furnished and installed by the erector. (Section 7 AISC 303–10 Code of Standard Practice)

It is the contractors responsibility to apply or observe all pertinent safety rules and regulations, as per OSHA standards as applicable.

The Contractor is responsible for the verification of all shipments received. Any "external" damage or shortages must be noted on all copies of the bill of lading and one copy is to be retained for your records. Failure to do so will make it impossible for the factory to honor any claim. NO EXCEPTIONS!!!

DESIGN LOADING

This structure is designed utilizing the loads indicated and applied by the : IBC 2018

It is the contractor's responsibility to confirm that these loads comply with the requirements of the local building department.

Specific loads : (See structural calculations and foundation reactions.)

20.00	PSF Live Load
No	LL Reduction Allowed
30	PSF Ground Snow Load
1.00	Thermal Factor (Ct)
1	Snow Exposure Factor (Ce)
120	MPH Wind Load Exposure C (If applicable)
2.000	PSF Dead Load (Metal Bldg. Weight – Purlins, Panels, Etc.)
1	PSF Collateral Load (Ceilings, Sprinklers, Etc.)
II – Normal	Risk Category I_s 1.0000 I_e 1.00

SEISMIC DATA :

1) Mapped Spectral Acceleration for Short Period, Ss 1.52
2) Site Coefficient, Fa 1.0000
3) Seismic Design Category = D
4) Seismic Coefficient = 1.01
5) Site Class = D
6) Basic Structural System and Seismic Resisting System Ordinary Moment Frame of Steel
7) Frames: R = 3.5000
8) Analysis Procedure = Equivalent Lateral Force

DRAWING INDEX

These Drawings are for :

☒ Construction ☐ Approval *
☐ Permit ☐ Anchor Bolts & Reactions

* Approval orders must be released for fabrication within thirty (30) calendar days after the submittal drawings are issued or they will be subject to any current price increases. Special attention should be given in approving dimensions and/or details. Please verify requested dimensions by indicating 'OK'.

FBC product approval numbers: FL19604 & FL19606

T&Z Consulting Services, LLC
Nevada COR No. 26501

Engineering Seal

This certification covers parts manufactured and delivered by the manufacturer only, and excludes parts such as doors, windows, foundation design and erection of the building. The buyer is responsible for ensuring all specified loads are in compliance with regulatory authorities.

PROFESSIONAL ENGINEER - STATE OF NEVADA
MINGQIAO ZHU
Exp: 06/30/2020
CIVIL
No. 15943

06/17/2019

PROFESSIONAL ENGINEER

DSN: MQZ	DWN: MKO
DET: MN	CHK: DP

REV:

REVISIONS

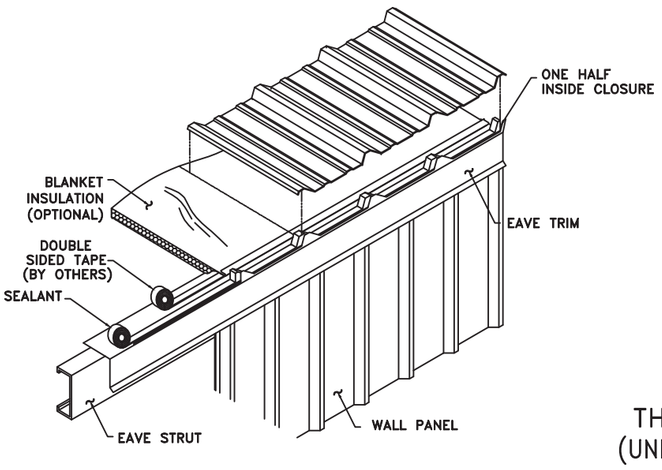
NO.	DATE

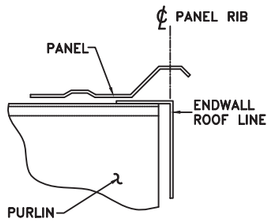
DRAWINGS COVER SHEET

CUSTOMER :
SCOTT HEBERT
11537 SITKA ST, RENO NV 89506
COUNTY : WASHOE COUNTY

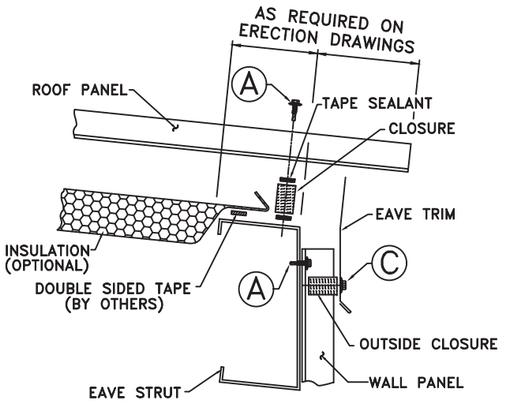
OLYMPIA STEEL BUILDINGS
400 ISLAND AVE
MCKEES ROCKS PA 15136

SCALE:	DATE:	JOB NO:	SHT. NO:
NOT TO SCALE	6/ 7/19	004989	CS–1



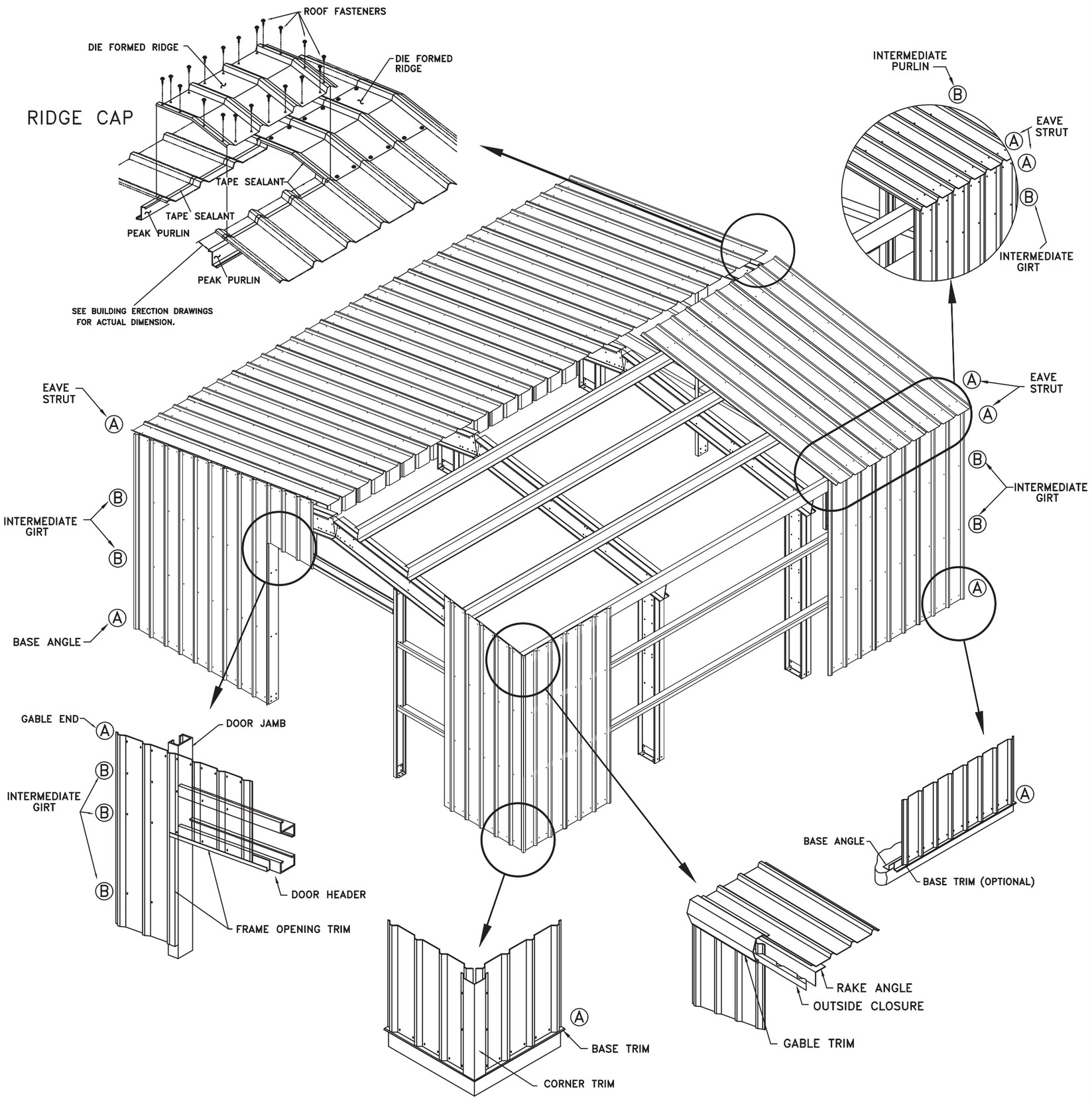


INSTALLATION OF
THE FIRST ROOF PANEL
(UNLESS NOTED ON DRAWINGS)

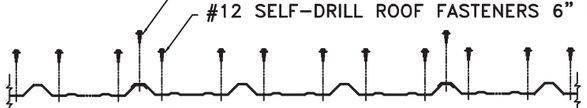


SECTION AT EAVE

FASTENER SCHEDULE

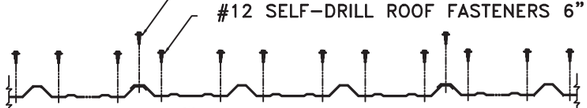


DETAIL: A



Ⓐ FASTENER PATTERN
AT BASE ANGLE, EAVE STRUT, RIDGE CAP
& INTERMEDIATE PANEL LAPS

DETAIL: B



Ⓑ FASTENER PATTERN
AT INTERMEDIATE PURLIN OR GIRT OR LINER PANEL

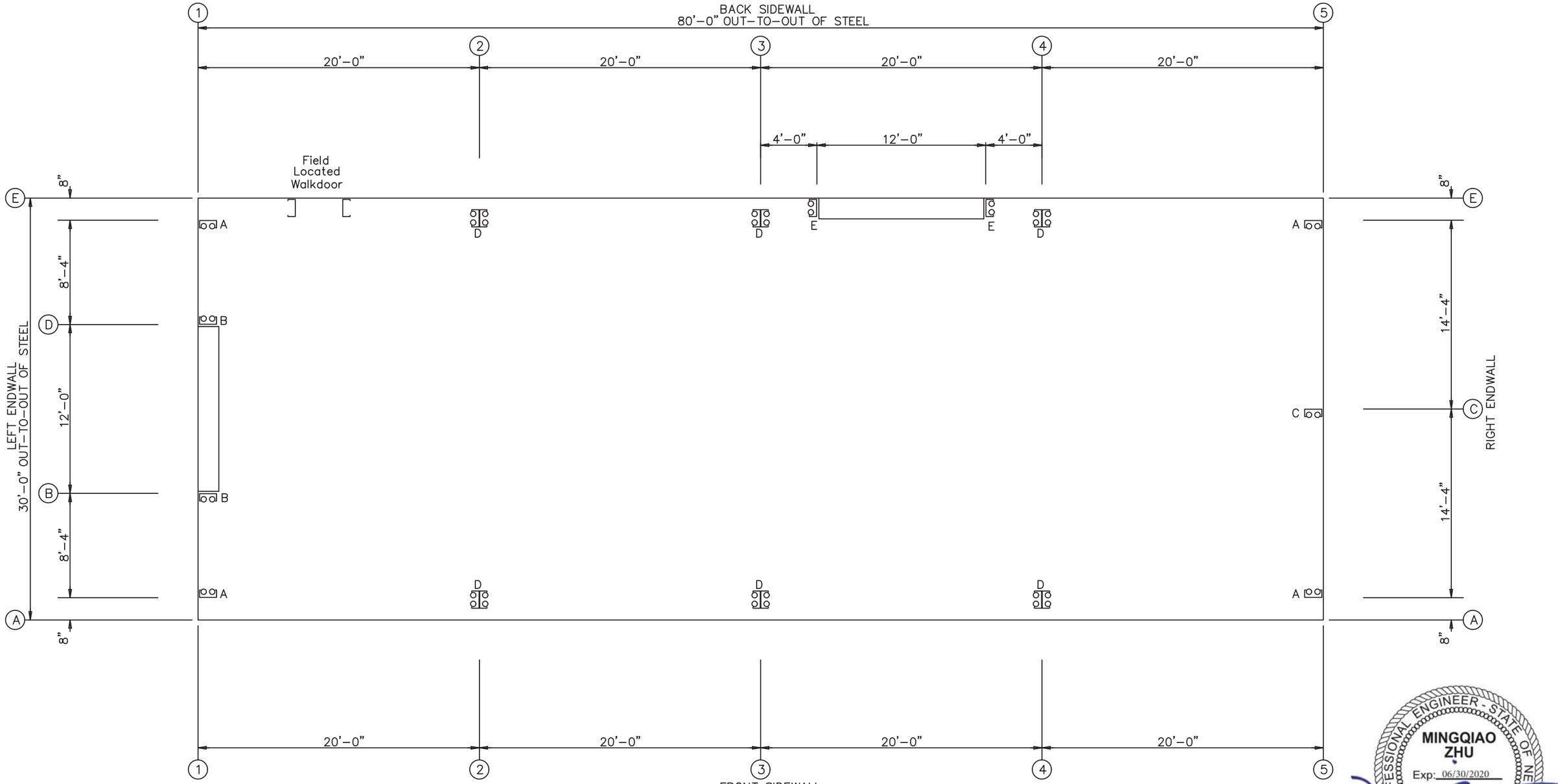
DETAIL: C

Ⓒ STITCH FASTENER
#14x7/8"
12" ON CENTER

DATE: 6/7/19

DWG NO: CS-2W

o Dia= 1/2"



FRONT SIDEWALL

ANCHOR BOLT PLAN

NOTE: All Base Plates @ 100'-0" (U.N.)

PROFESSIONAL ENGINEER - STATE OF NEVADA
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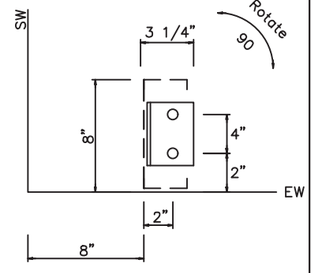
06/17/2019

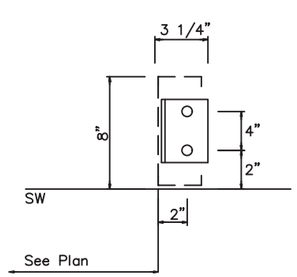
OLYMPIA STEEL BUILDINGS		Customer: SCOTT HEBERT	
MCKEES ROCKS PA 15136		RENO NV 89506	
Drafter: MKO	Date: 6/ 7/19	Designer: MQZ	Date: 6/ 7/19
Detailer: MN	Date: 6/ 7/19	Sales ID:	Factory ID: 004989
Checker: DP	Date: 6/ 7/19		
ANCHOR BOLT PLAN			Sht E1 of 11

Dia= 1/2"



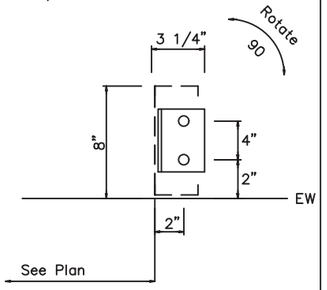
DETAIL A

Dia= 1/2"



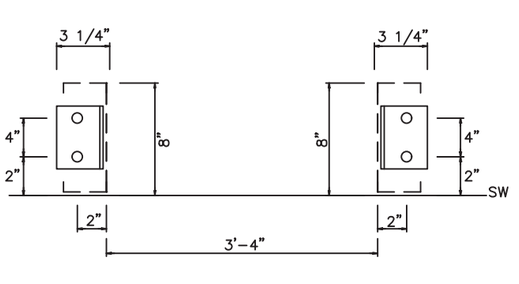
DETAIL E

Dia= 1/2"



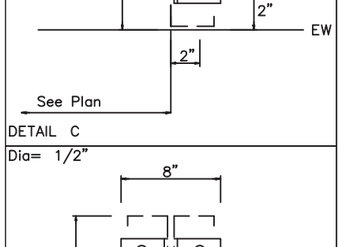
DETAIL B

Dia= 1/2"



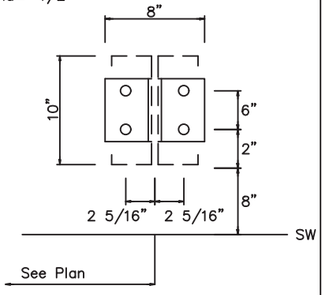
DETAIL OF FIELD LOCATED WALKDOOR

Dia= 1/2"



DETAIL C

Dia= 1/2"



DETAIL D

LOAD CASES

WIND_L1 = WIND LOAD FROM LEFT CASE 1
WIND_R1 = WIND LOAD FROM RIGHT CASE 1
WIND_LN1 = LONGITUDINAL WIND CASE 1

SEISMIC_L = SEISMIC LOAD FROM LEFT
SEISMIC_R = SEISMIC LOAD FROM RIGHT
SEISMICLN = LONGITUDINAL SEIMIC LOAD

F1UNB_SL_L = FRAME 1 UNBALANCED SNOW LEFT SIDE
F1UNB_SL_R = FRAME 1 UNBALANCED SNOW RIGHT SIDE
F1CRANE 1 = FRAME 1 CRANE LOAD IN POSITION 1

DRIFT = SNOW DRIFT LOAD
SLIDE = SLIDE SNOW LOAD

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	=	30
Length (ft)	=	80
Eave Height (ft)	=	16 / 16
Roof Slope	=	2.0:12 / 2.0:12
Dead Load (psf)	=	2.000
Collateral Load (psf)	=	1
Roof Live Load (psf)	=	20.00
Roof Snow Load (psf)	=	21
Wind Speed(mph)	=	120
Wind Code	=	IBC 2018
Wind Exposure	=	C
Closed/Open	=	Closed
Importance – Seismic	=	1.00
Importance – Wind	=	1.00
Seismic Design Category	=	D
Seismic Coeff (Fa*Ss)	=	1.515

Load Combinations

DL+CL+(LL or SL)
DL+CL+0.6WL
DL+CL+0.75(0.6)WL+0.75(LL or SL)
DL+0.75(0.7SEIS)+0.75(LL or SL)
0.6DL+0.6WL
0.6DL+0.7SEIS

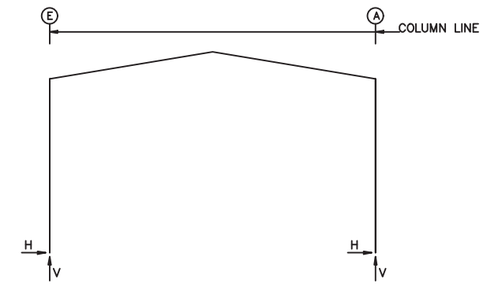
GENERAL NOTES

1. FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF THE BUILDING MANUFACTURER.
2. THE BUILDING REACTION DATA, REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION.
3. THE SPECIFIED ANCHOR BOLT DIAMETER ASSUMES ASTM A307. ANCHOR BOLT MATERIAL OF EQUAL DIAMETER MEETING OR EXCEEDING THE STRENGTH REQUIREMENTS SET FORTH ON THESE DRAWINGS MAY BE UTILIZED AT THE DISCRETION OF THE FOUNDATION DESIGN ENGINEER.
4. ANCHOR BOLTS TO BE SUPPLIED BY OTHERS. ANCHOR BOLT EMBEDMENT LENGHT SHALL BE DETERMINED BY THE FOUNDATION ENGINEER.
5. ANCHOR BOLT PROJECTION ABOVE CONCRETE FINISHED SURFACE TO BE 3" UNLESS OTHERWISE NOTED BY FOUNDATION DESIGNER.
6. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLERANCE OF +/– 1/8" IN ELEVATION AND LOCATION.
7. THE ANCHOR BOLT LOCATIONS PROVIDED BY THE METAL BUILDING MANUFACTURER MAY NOT SATISFY ANCHOR BOLT CONCRETE EDGE DISTANCE REQUIREMENTS DEPENDING ON THE DETAILS OF FOUNDATION DESIGN. IT IS THE RESPONSIBILITY OF THE FOUNDATION DESIGN ENGINEER TO MAKE SURE THAT SUFFICIENT CONCRETE EDGE DISTANCE IS PROVIDED IN THE FOUNDATION DESIGN.
8. MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. WE WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.
9. THIS DRAWING IS NOT TO SCALE.

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel Shear (lb/ft) Wind	Seis
L_EW	1						102	93
F_SW	A						20	12
R_EW	5						61	55
B_SW	E						24	15

FRAME LINES: 2 3 4



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Dia	Base Plate (in) Width	Length	Thick	Grout (in)
2*	E	4	0.500	8.000	7.000	0.375	0.0
2*	A	4	0.500	8.000	7.000	0.375	0.0

2* Frame lines: 2 3 4

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
2*	E	0.2	0.9	0.1	0.3	1.3	6.0	1.3	6.3	−3.8	−8.1	2.4	−3.5
2*	A	−0.2	0.9	−0.1	0.3	−1.3	6.0	−1.3	6.3	−2.4	−3.5	3.8	−8.1

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
2*	E	−4.7	−5.2	1.6	−0.6	1.6	−6.2	1.2	−5.3	−0.8	−0.9	0.8	0.9
2*	A	−1.6	−0.6	4.7	−5.2	−1.2	−5.3	−1.6	−6.2	−0.8	0.9	0.8	−0.9

Frame Line	Column Line	F1UNB_SL_L Horiz	F1UNB_SL_L Vert	F1UNB_SL_R Horiz	F1UNB_SL_R Vert
2*	E	1.1	6.6	1.1	3.6
2*	A	−1.1	3.6	−1.1	6.6

2* Frame lines: 2 3 4

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horz	Wind Suct Horz	Wind Long1 Vert	Wind Long2 Vert
1	E	0.1	0.0	0.5	0.5	−0.7	−0.3	−0.2	0.2	−0.8	1.0	−0.8	−0.4
1	D	0.4	0.1	2.5	2.7	−3.5	−2.5	−2.6	−1.6	−1.9	2.1	−3.4	−2.3
1	B	0.4	0.1	2.5	2.7	−2.5	−3.5	−1.6	−2.6	−1.9	2.1	−2.3	−3.4
1	A	0.1	0.0	0.5	0.5	−0.3	−0.7	0.2	−0.2	−0.8	1.0	−0.4	−0.8

Frm Line	Col Line	Seis Left Vert	Seis Right Vert	E1UNB_SL_L Horz	E1UNB_SL_L Vert	E1UNB_SL_R Horz	E1UNB_SL_R Vert
1	E	0.2	0.5	0.0	0.6	0.0	0.0
1	D	−0.2	−0.4	0.0	3.4	0.0	1.3
1	B	−0.4	−0.2	0.0	1.3	0.0	3.4
1	A	0.5	0.2	0.0	0.0	0.0	0.6

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horz	Wind Suct Horz	Wind Long1 Vert	Wind Long2 Vert
5	A	0.2	0.1	1.6	1.7	−2.2	−1.4	−1.3	−0.5	−1.3	1.5	−2.3	−1.4
5	C	0.4	0.1	2.9	3.1	−3.5	−3.5	−2.4	−2.4	−2.9	3.2	−3.3	−3.3
5	E	0.2	0.1	1.6	1.7	−1.4	−2.2	−0.5	−1.3	−1.3	1.5	−1.4	−2.3

Frm Line	Col Line	Seis Left Vert	Seis Right Vert	E2UNB_SL_L Horz	E2UNB_SL_L Vert	E2UNB_SL_R Horz	E2UNB_SL_R Vert
5	A	0.1	0.3	0.0	2.0	0.0	0.5
5	C	−0.4	−0.4	0.0	2.7	0.0	2.7
5	E	0.3	0.1	0.0	0.5	0.0	2.0

ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Dia	Base Plate (in) Width	Length	Thick	Grout (in)
1	E	2	0.500	3.250	6.000	0.375	0.0
1	D	2	0.500	3.250	6.000	0.375	0.0
1	B	2	0.500	3.250	6.000	0.375	0.0
1	A	2	0.500	3.250	6.000	0.375	0.0
5	A	2	0.500	3.250	6.000	0.375	0.0
5	C	2	0.500	3.250	6.000	0.375	0.0
5	E	2	0.500	3.250	6.000	0.375	0.0

06/17/2019

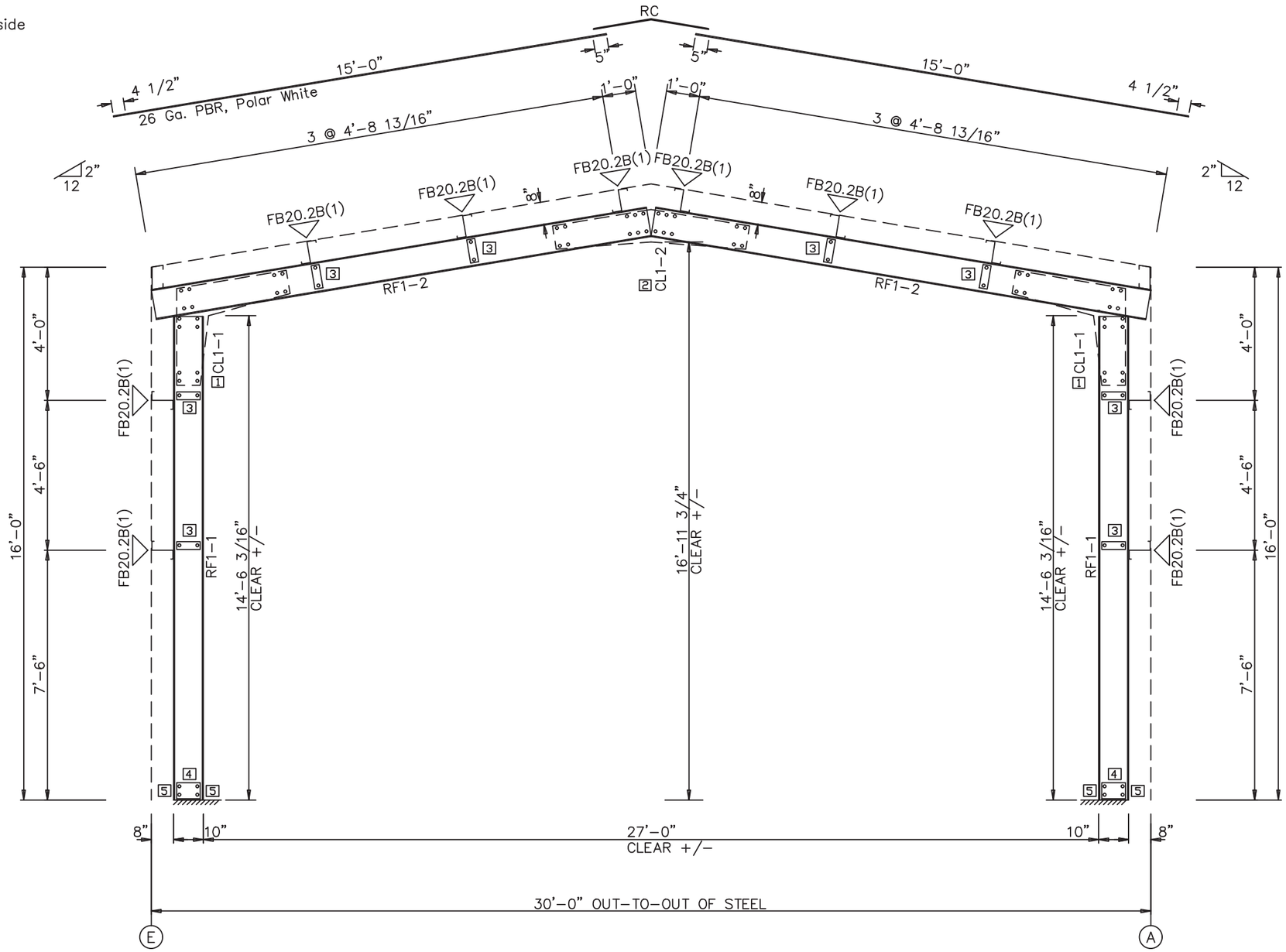
OLYMPIA STEEL BUILDINGS		Customer: SCOTT HEBERT	
MCKEES ROCKS PA 15136		RENO NV 89506	
Drafter: MKO	Date: 6/ 7/19	Designer: MQZ	Date: 6/ 7/19
Detailer: MN	Date: 6/ 7/19	Sales ID:	Factory ID: 004989
Checker: DP	Date: 6/ 7/19		
ANCHOR BOLT DETAILS & REACTIONS			Sht E2 of 11

SPLICE BOLT TABLE						
Mark	Qty Top	Bot	Int	Type	Dia	Length
CL1–1	8	8	0	A325	0.500	2.00
CL1–2	10	10	0	A325	0.500	2.00

▽FLANGE BRACES: Both Sides(U.N.)
FB20.2B(1): 20.2=length(in), (1)=one side
B – L2X2X10G

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
RF1–1	10x55D12	14'–7 1/16"
RF1–2	10x55D12	15'–0 13/16"

CONNECTION PLATES	
☐ ID	Mark/Part
1	CL1–1
2	CL1–2
3	CLC101
4	CLC100
5	CLC003



RIGID FRAME ELEVATION: FRAME LINE 2 3 4

06/17/2019

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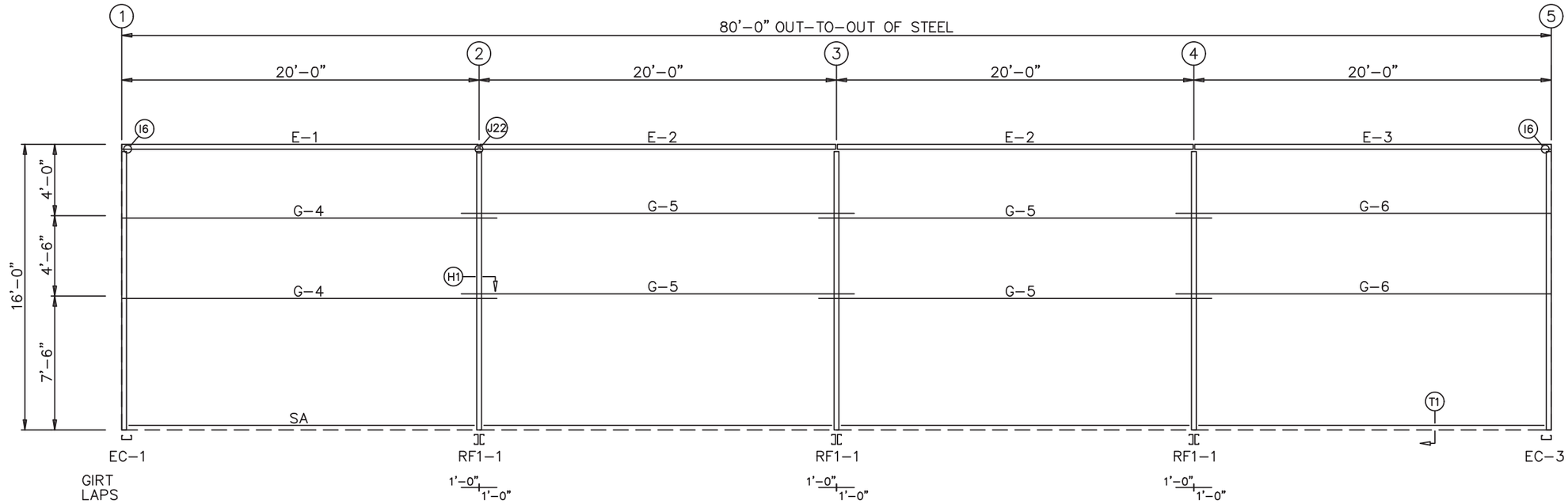
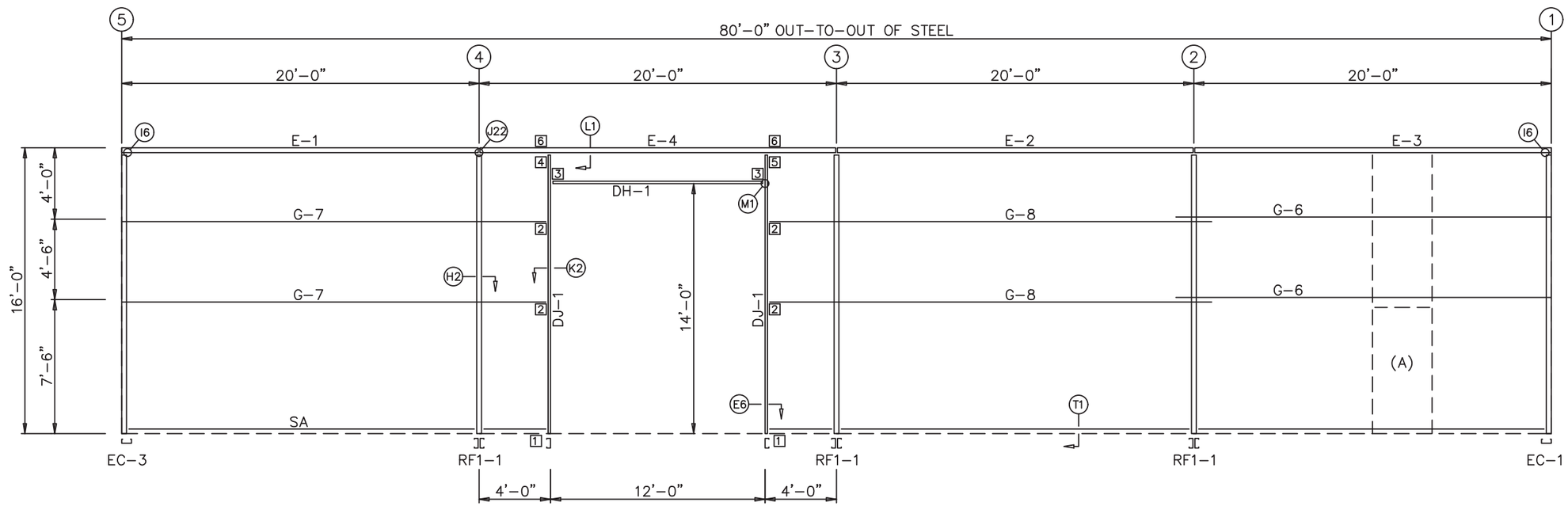
Checker: DP	Date: 6/ 7/19		
RIGID FRAME ELEVATION			Sht E3 of 11

MEMBER TABLE FRAME LINE A & E		
MARK	PART	LENGTH
DJ-1	8x275C16	15'-3 3/8"
DJ-2	8x275C16	15'-3 3/8"
DH-1	8x275C16	11'-11 1/2"
DH-2	8x275C16	3'-3 1/2"
E-1	8x275E16	19'-11 1/4"
E-2	8x275E16	19'-11 1/2"
E-3	8x275E16	19'-11 1/4"
E-4	8x275E16	19'-11 1/2"
G-4	8x25Z16	20'-11 1/2"
G-5	8x25Z16	22'-0"
G-6	8x25Z16	20'-11 1/2"
G-7	8x25Z16	23'-7"
G-8	8x25Z16	24'-7 1/2"

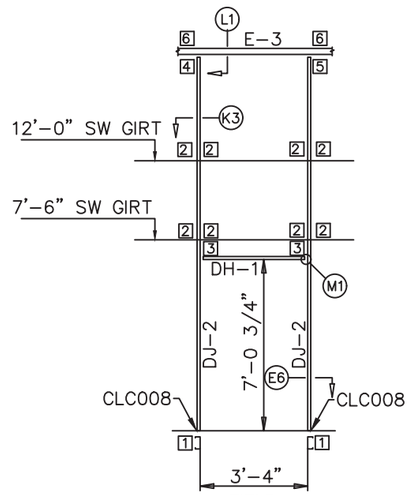
CONNECTION PLATES FRAME LINE A & E	
ID	MARK/PART
1	CLC008
2	CLC062
3	CLC025
4	CLC213
5	CLC212
6	SA



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL FRAMING: FRAME LINE E



FIELD
3471 WALKDOOR
FRAMED OPENING DETAIL
(1 REQUIRED)
(8" GIRT)
(A)

06/17/2019

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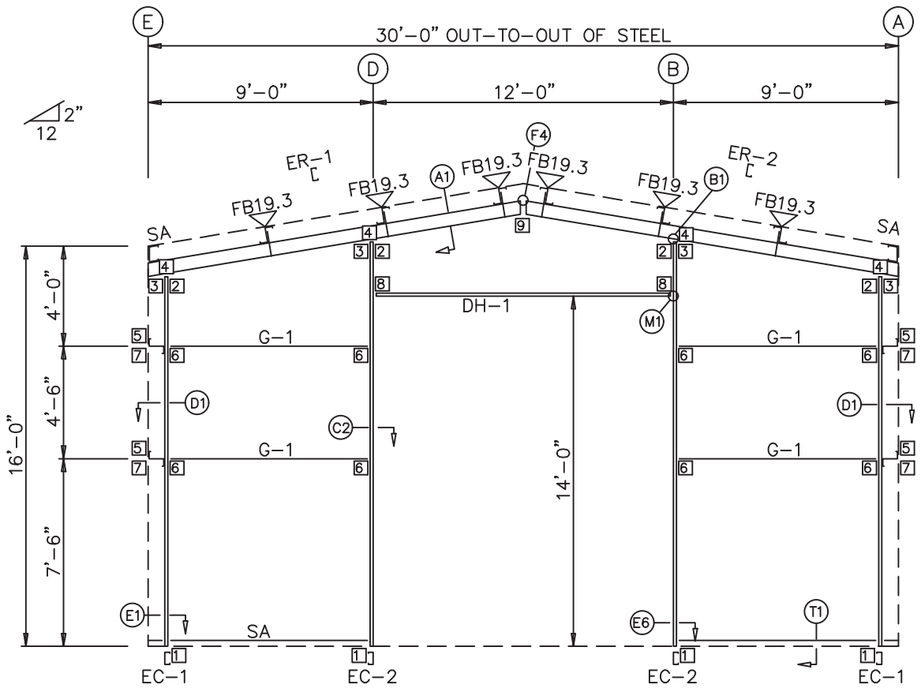
SIDEWALL FRAMING			Sht E4 of 11

BOLT TABLE FRAME LINE 1 & 5				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	4	A325T	1/2"	2"
ER-3/ER-4	4	A325T	1/2"	2"
Columns/Raf	8	A325T	1/2"	2"

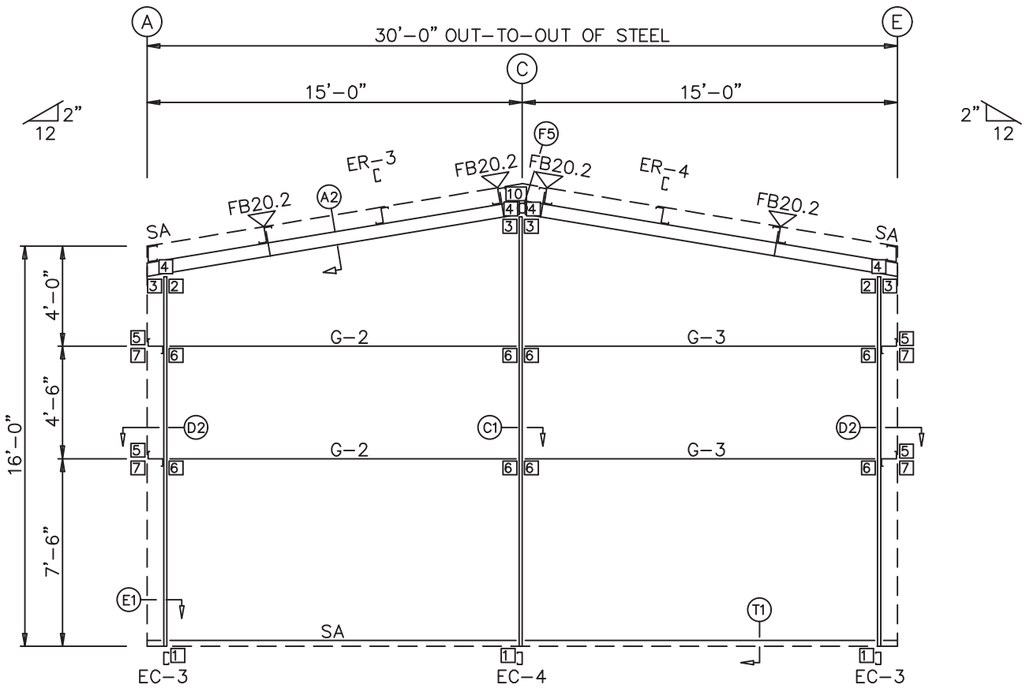
MEMBER TABLE FRAME LINE 1 & 5		
MARK	PART	LENGTH
EC-1	8x275C16	14'-8 1/8"
EC-2	8x275C15	16'-0 3/4"
EC-3	8x275C16	14'-6 1/16"
EC-4	8x275C12	16'-10 3/4"
ER-1	8x275C14	15'-1"
ER-2	8x275C14	15'-1"
ER-3	10x38C14	15'-0 11/16"
ER-4	10x38C14	15'-0 11/16"
DH-1	8x275C16	11'-11 1/2"
G-1	4x25Z18	7'-7"
G-2	8x25Z16	13'-7"
G-3	8x25Z16	13'-11"

FLANGE BRACE TABLE FRAME LINE 1 & 5			
▽ID	QUAN	MARK	LENGTH
1	6	FB19.3	1'-7 1/4"
2	4	FB20.2	1'-8 1/4"

CONNECTION PLATES FRAME LINE 1 & 5	
□ID	MARK/PART
1	CLC008
2	CLC081
3	CLC083
4	CLC089
5	CLC054
6	CLC062
7	CLC053
8	CLC025
9	CLC122
10	SCL-1



ENDWALL FRAMING: FRAME LINE 1



ENDWALL FRAMING: FRAME LINE 5

06/17/2019

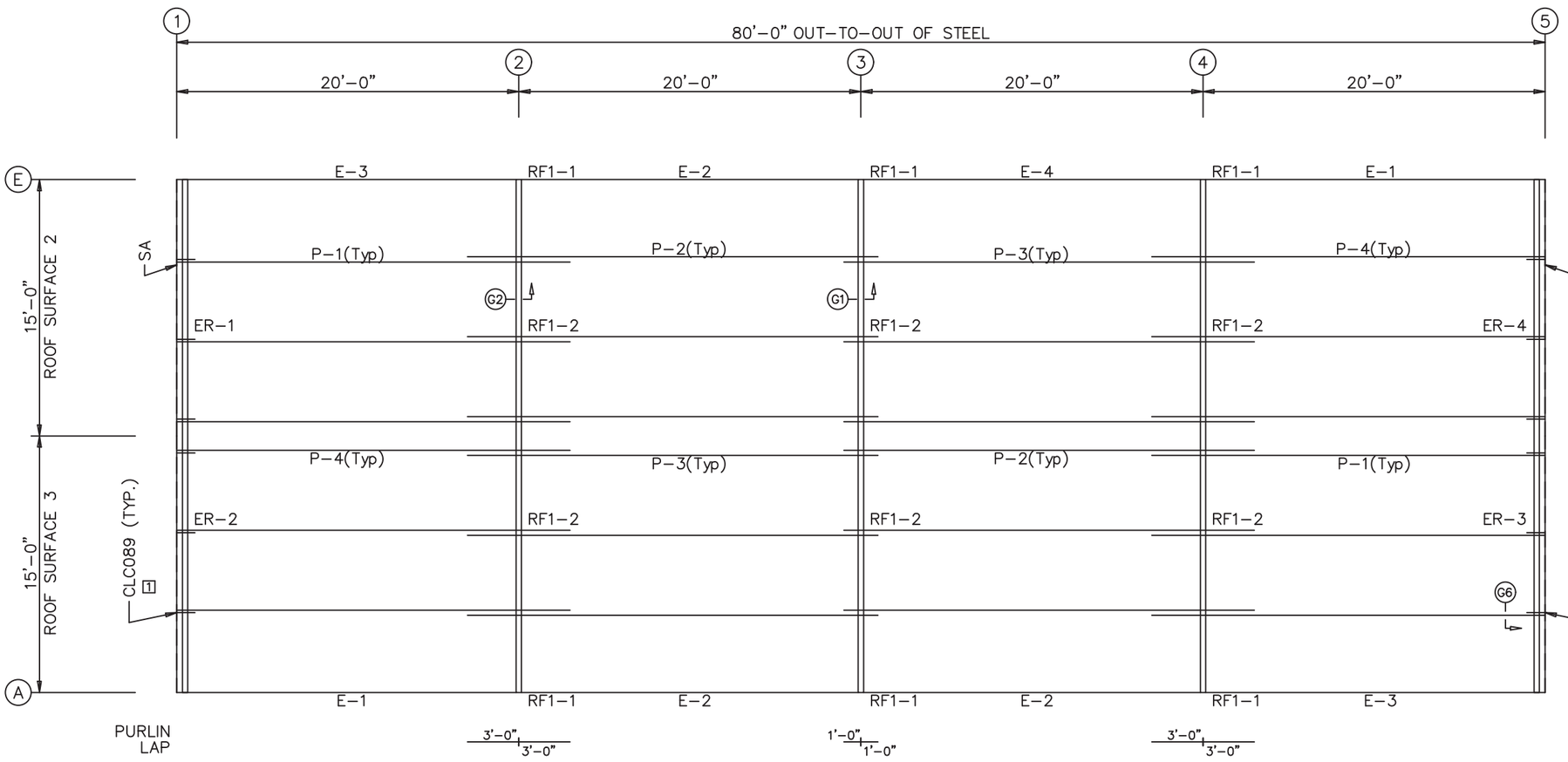
GENERAL NOTES:

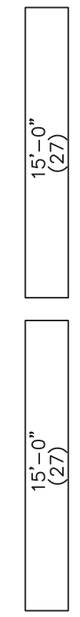
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. WE WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

OLYMPIA STEEL BUILDINGS		Customer: SCOTT HEBERT	
MCKEES ROCKS PA 15136		RENO NV 89506	
Drafter: MKO	Date: 6/ 7/19	Designer: MQZ	Date: 6/ 7/19
Detailer: MN	Date: 6/ 7/19	Sales ID:	Factory ID: 004989
Checker: DP	Date: 6/ 7/19		
ENDWALL FRAMING			Sht E5 of 11

MEMBER TABLE ROOF PLAN		
MARK	PART	LENGTH
P–1	8x25Z16	22'–11 1/2"
P–2	8x25Z16	24'–0"
P–3	8x25Z16	24'–0"
P–4	8x25Z16	22'–11 1/2"
E–1	8x275E16	19'–11 1/4"
E–2	8x275E16	19'–11 1/2"
E–3	8x275E16	19'–11 1/4"
E–4	8x275E16	19'–11 1/2"

CONNECTION PLATES ROOF PLAN	
☐ID	MARK/PART
1	CLC089





ROOF FRAMING PLAN

ROOF SHEETING

PANELS: 26 Ga. PBR
Polar White

PROFESSIONAL ENGINEER - STATE OF NEVADA
MINGQIAO ZHU
Exp: 06/30/2020
CIVIL
No. 15943

06/17/2019

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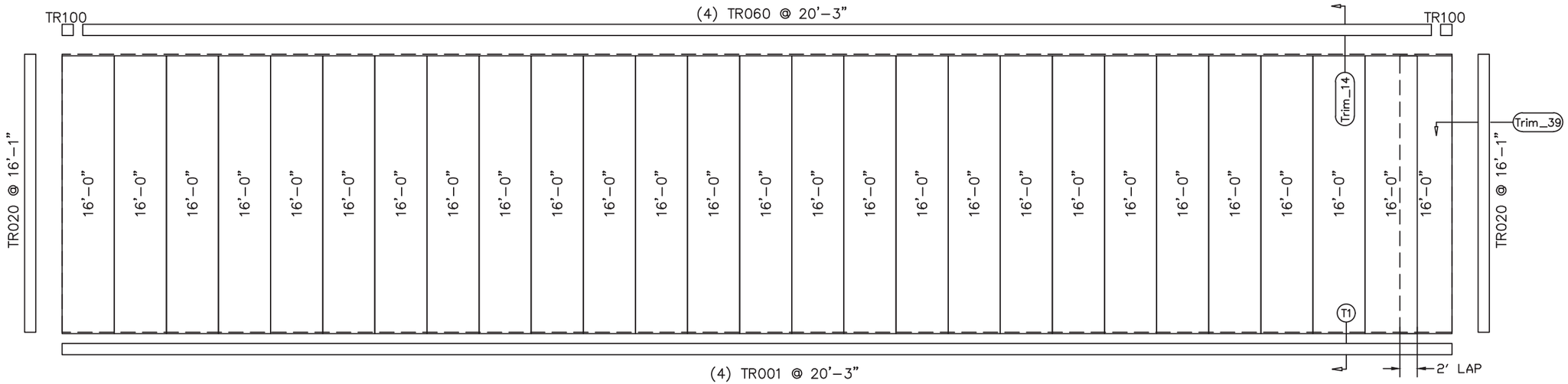
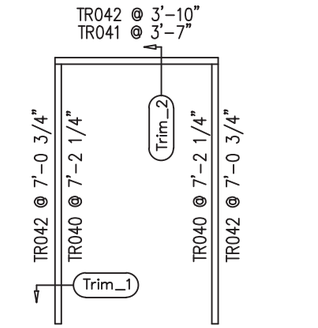
Checker: DP	Date: 6/ 7/19		
ROOF FRAMING PLAN			Sht E6 of 11

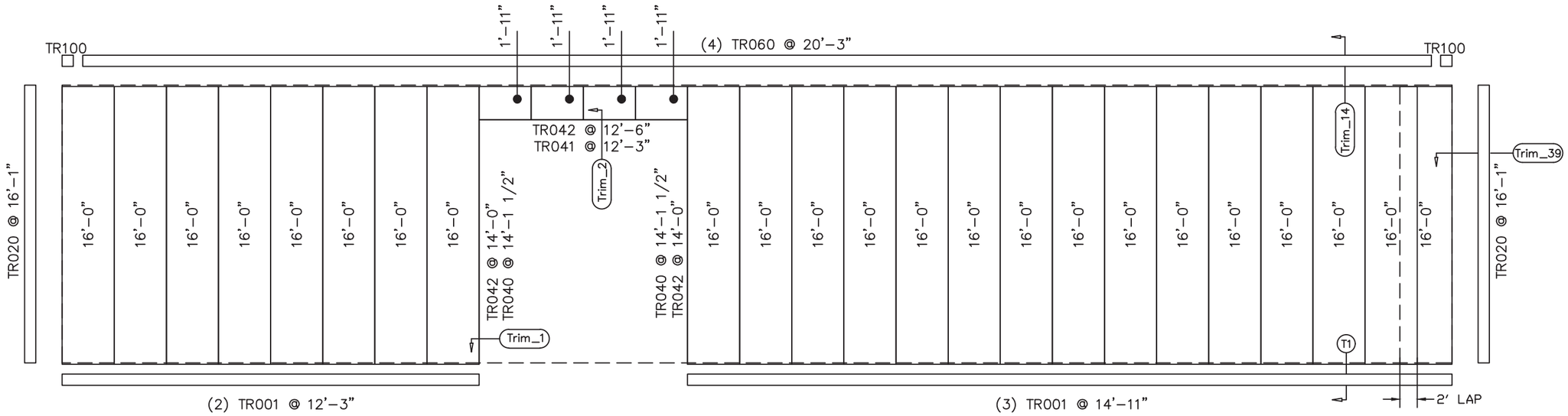


SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. PBR – Brite Red



TRIM DETAIL LAYOUT
FOR 3'-4" x 7'-0 3/4" WALKDOOR
(1 REQD.)



SIDEWALL SHEETING & TRIM: FRAME LINE E

PANELS: 26 Ga. PBR – Brite Red

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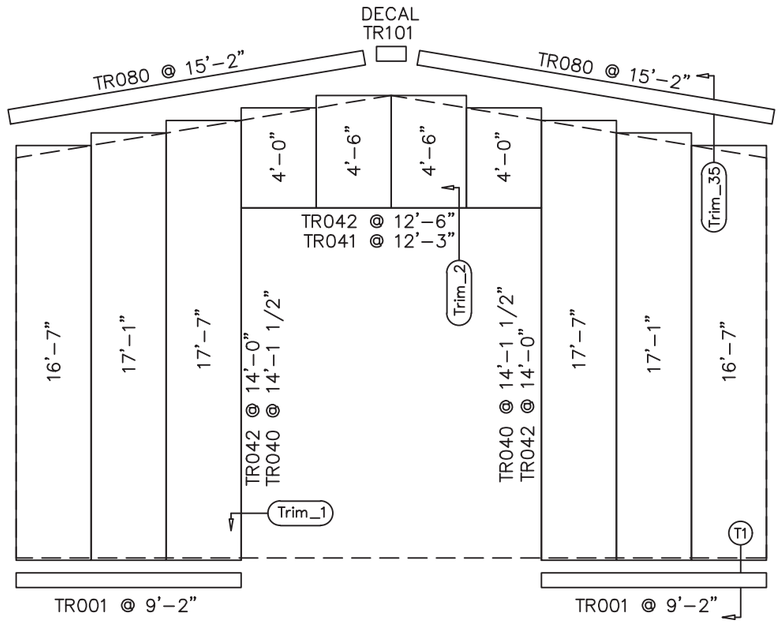
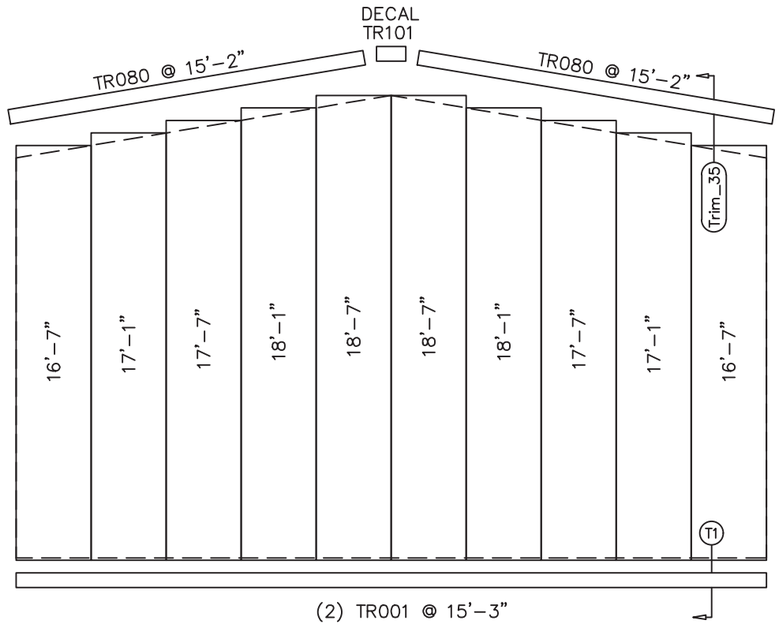
Detailer: MN	Date: 6/ 7/19	Sales ID:	Factory ID: 004989
Checker: DP	Date: 6/ 7/19		
SIDEWALL SHEETING			Sht E7 of 11



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. PBR – Brite Red



ENDWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 Ga. PBR – Brite Red

06/17/2019

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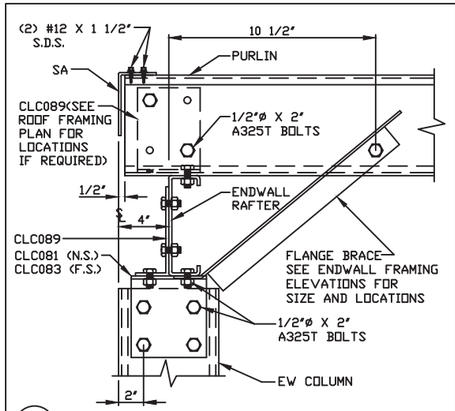
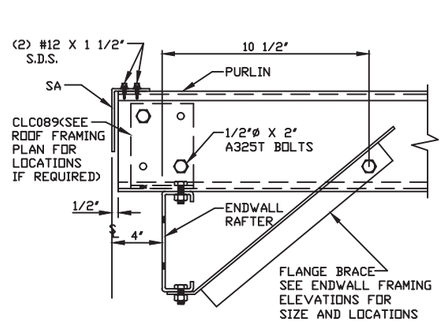
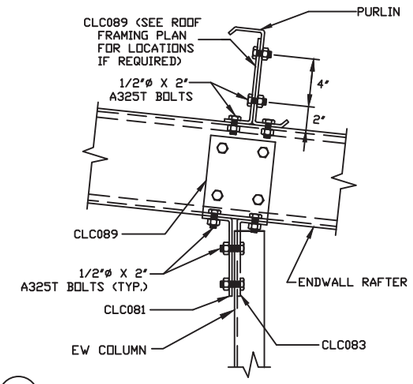
Checker: DP	Date: 6/ 7/19		
ENDWALL SHEETING			Sht E8 of 11



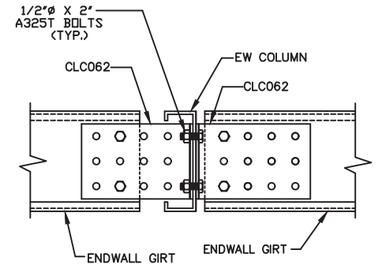
A1 PURLIN TO ENDWALL RAFTER



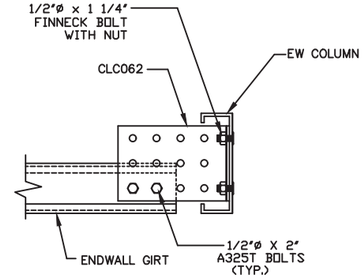
A2 PURLIN TO ENDWALL RAFTER



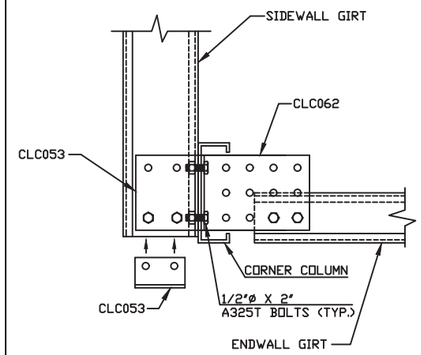
B1 EW COLUMN TO ENDWALL RAFTER



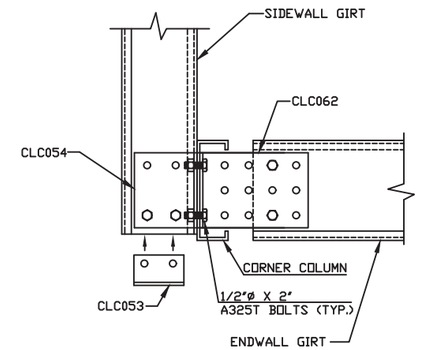
C1 ENDWALL COLUMN GIRT CONNECTION



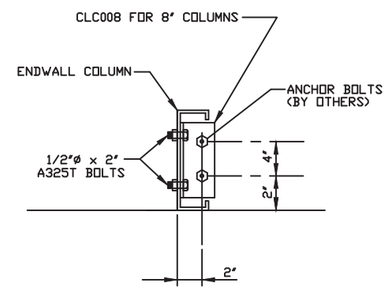
C2 ENDWALL COLUMN GIRT CONNECTION



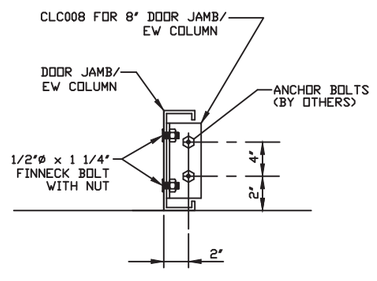
D1 CORNER COLUMN GIRT CONNECTION



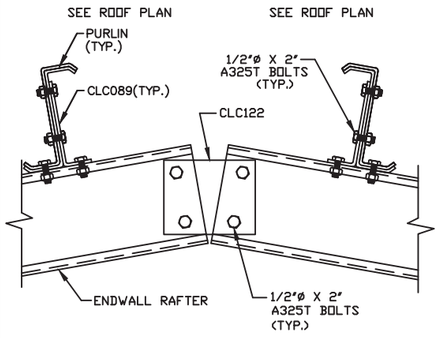
D2 CORNER COLUMN GIRT CONNECTION



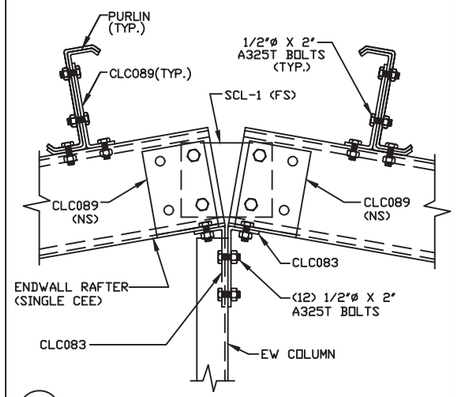
E1 ENDWALL COLUMN BASE SECTION



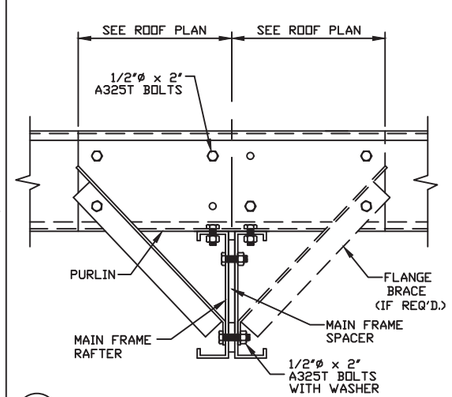
E6 EW COLUMN/DOOR JAMB AT OPENING BASE SECTION



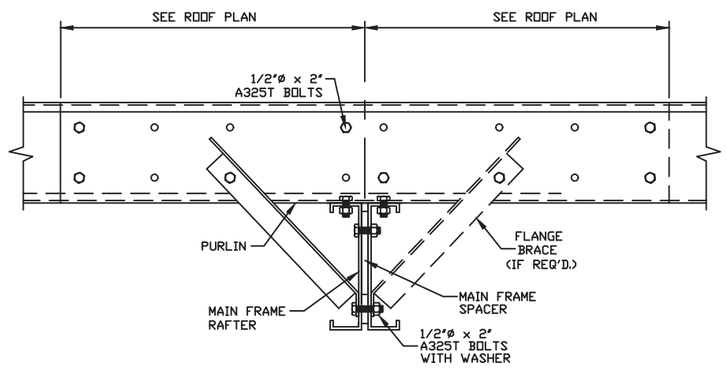
F4 ENDWALL RAFTER PEAK CONNECTION



F5 ENDWALL RAFTER PEAK CONNECTION



G1 PURLIN TO MAIN FRAME RAFTER



G2 PURLIN TO MAIN FRAME RAFTER WITH 3' LAP

NOTES: 1.) ALL SCREWS ARE WITH WASHERS UNLESS NOTED.

06/17/2019

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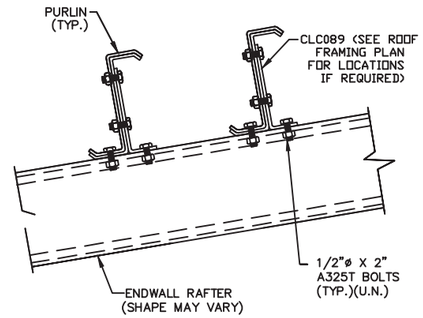
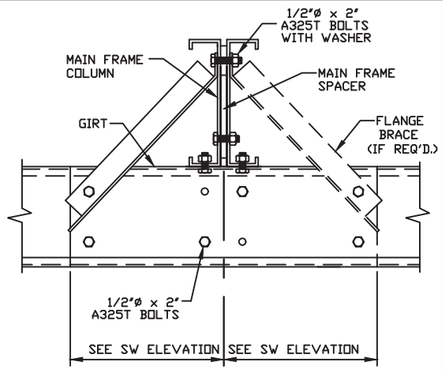
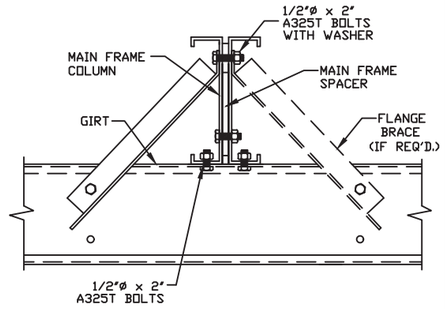
Checker: DP	Date: 6/ 7/19		
DETAIL DRAWINGS			Sht E9 of 11



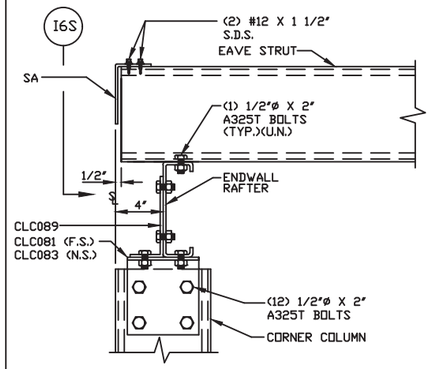
G6 PURLIN CLIP DETAIL



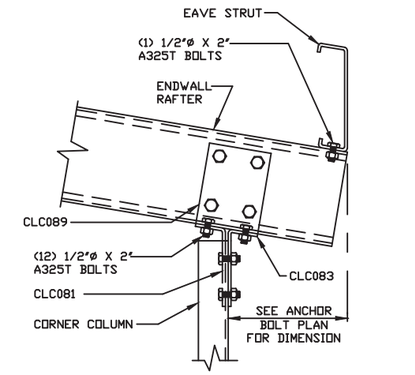
H1 GIRT TO MAIN FRAME COLUMN WITH LAP



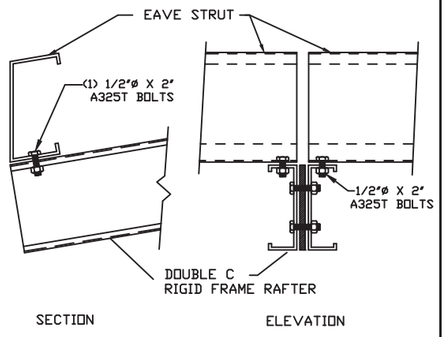
H2 GIRT TO MAIN FRAME COLUMN WITHOUT LAP



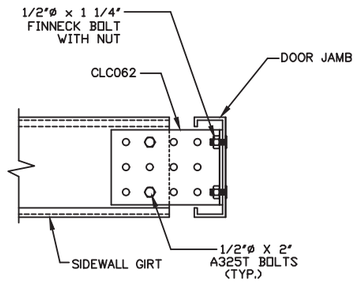
I6 CORNER COLUMN TO ENDWALL RAFTER



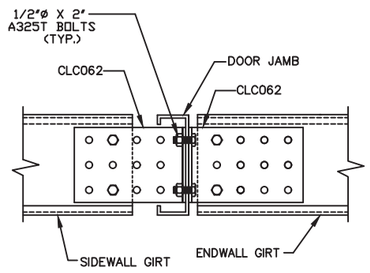
I6S CORNER COLUMN TO ENDWALL RAFTER



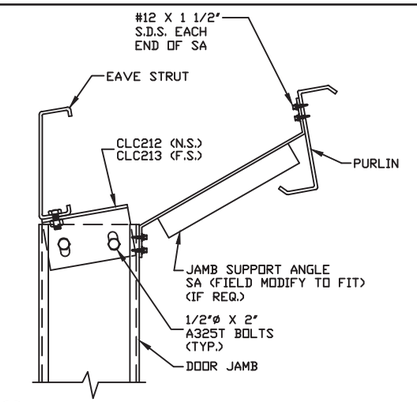
J22 EAVE STRUT TO MAIN FRAME



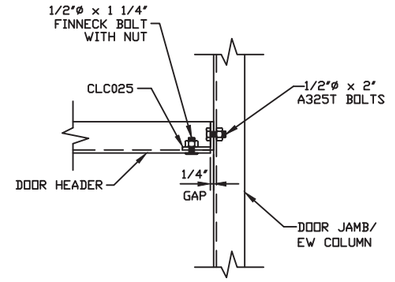
K2 DOOR JAMB TO GIRT CONNECTION



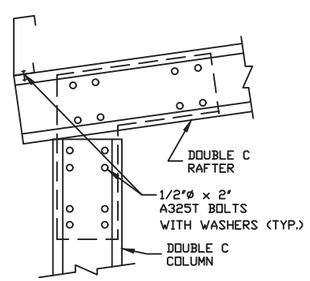
K3 DOOR JAMB TO GIRT CONNECTION



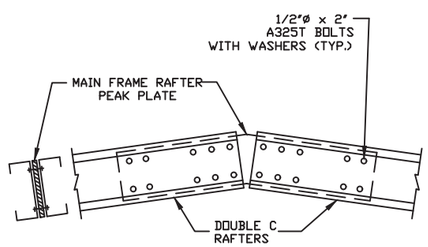
L1 DOOR JAMB TO EAVE STRUT



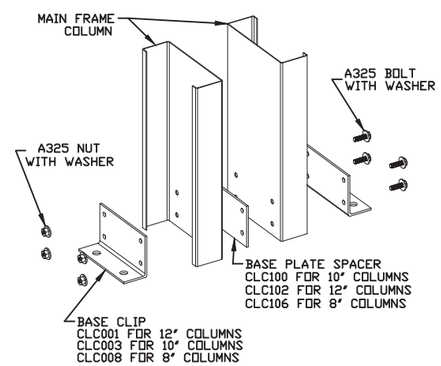
M1 HEADER TO DOOR JAMB/EW COLUMN CONNECTION



U11 MAIN FRAME COLUMN TO MAIN FRAME RAFTER



U12 MAIN FRAME RAFTER PEAK CONNECTION



SIDEWALL COLUMN BASE CONNECTION

06/17/2019

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Detailer: MN	Date: 6/ 7/19	Sales ID:	Factory ID: 004989
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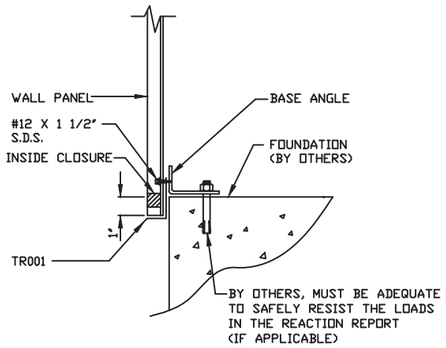
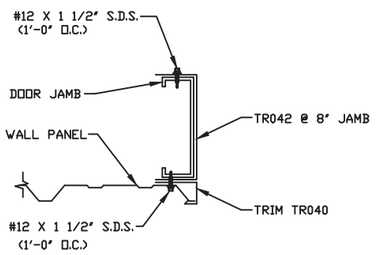
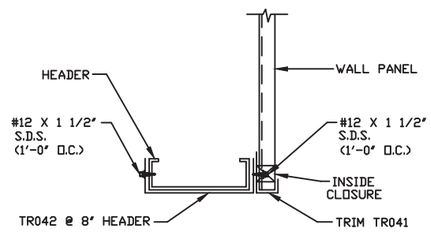
DETAIL DRAWINGS			Sht E10 of 11



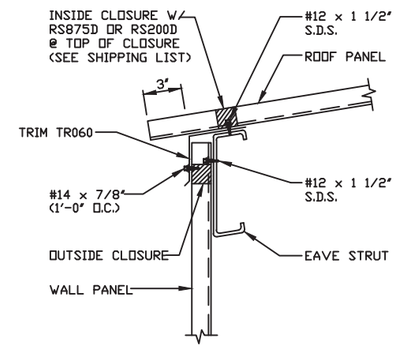
T1 BASE TRIM SECTION



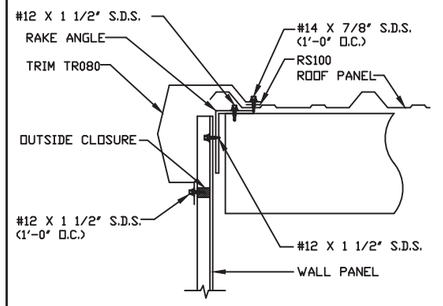
Trim_1 DOOR JAMB TRIM



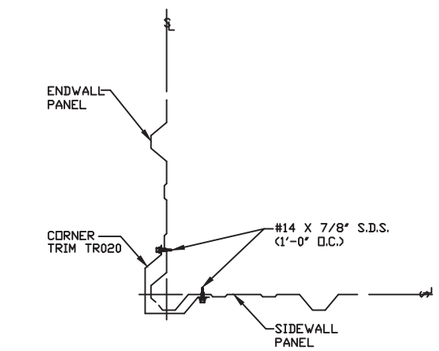
Trim_2 HEADER TRIM



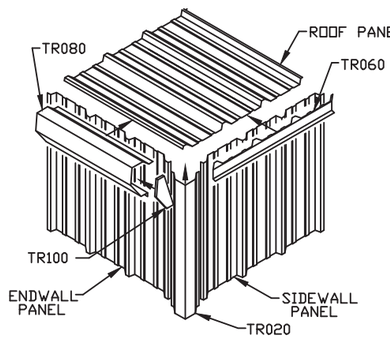
Trim_14 EAVE TRIM SECTION



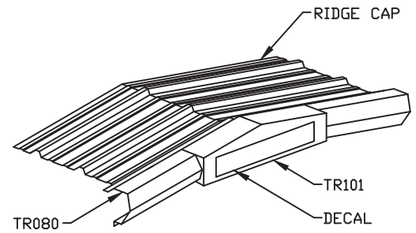
Trim_35 RAKE TRIM SECTION



Trim_39 CORNER TRIM SECTION



BUILDING CORNER DETAIL



PEAK DETAIL

06/17/2019

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TRIM DRAWINGS			Sht E11 of 11